Water Commissioner Training

Title 85-5
Montana Code Annotated

Helena, Montana April 7-8, 2016

Department of Natural Resources and Conservation (DNRC)





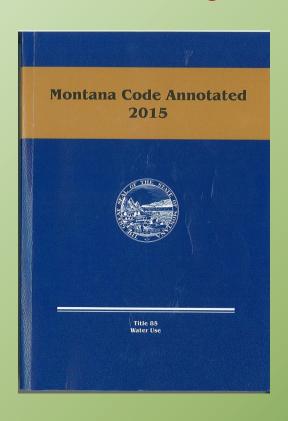
Handouts

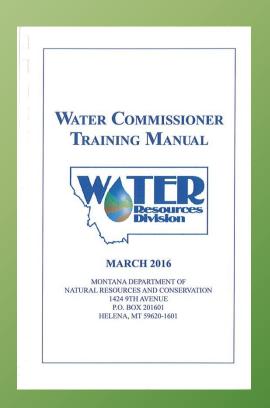
- ➤ Water Commissioner Training Manual (2016)
- ➤ Water Rights in Montana (2014)
- ➤ Irrigation Water Measurement (Wyoming Pocket Guide)
- > Problem Sets
- > "Water" Trivia!
- ➤ Additional Handouts

Speakers

- Peter Fritsch, Water Master, Montana Water Court
- Mike McLane, Montana Dept. Fish, Wildlife, and Parks
- Jim Ferch, New Appropriations Manager, DNRC Water Resources Division
- Matt Norberg, Hydrologist, DNRC Water Resources Division
- Jim Beck, Retired DNRC Water Resource Engineer
- John Conners, DNRC Regional Office Engineer
- Mike Roberts, Hydrologist, DNRC Water Resources Division

Why do we train Water Commissioners? 1989 Montana Legislature MCA 85-5-111





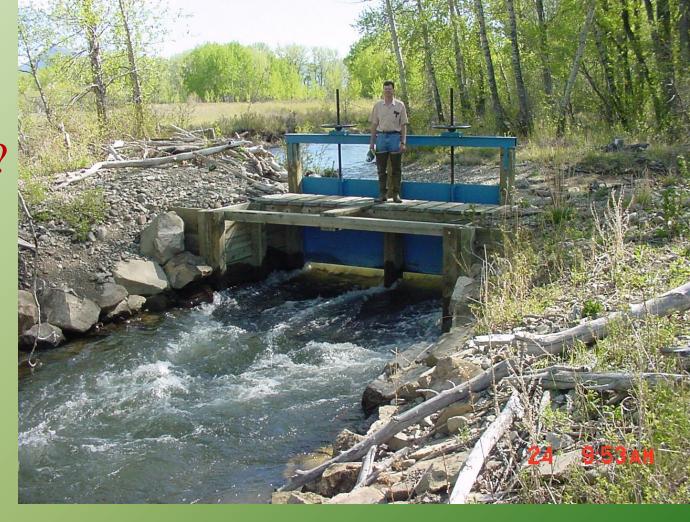
Heightened awareness of water management:

- adjudication Water Court Decrees
- drought
- water right hearings



Water Mediation Training

MCA 85-5-110 MCA 85-5-111 What is a Water Commissioner?



An appointee of the District Court responsible for the measurement and delivery of water based upon the priority of water rights for a specific stream, ditch, reservoir, or other watercourse.

Ditch Rider? Dam Tender? Water Commissioner? Mediator?

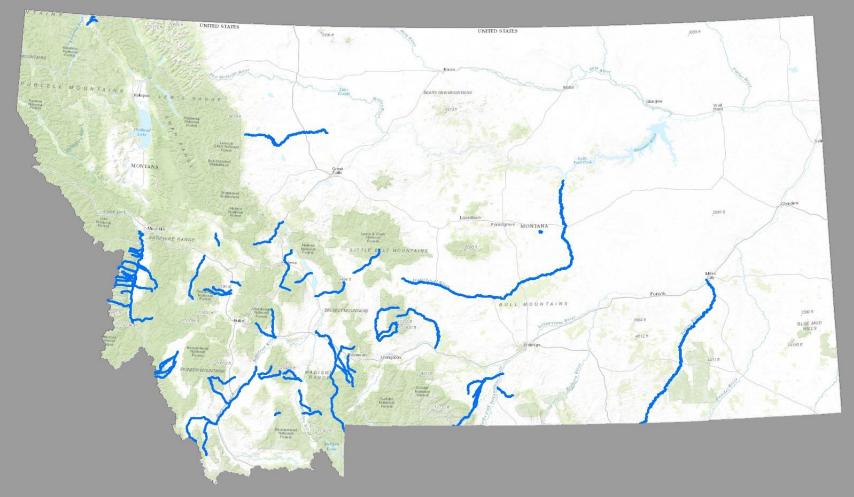
MCA 85-5-101 Applies to any stream, ditch, watercourse, spring, lake, reservoir, or other source of supply which has been determined by a decree of a court of competent jurisdiction (including temporary preliminary, preliminary, and final decrees).







Sources With Active Water Commissioners 2015



Water Supply Organizations

Irrigation Districts

Quasi-governmental organizations authorized by Montana District Courts. Many are associated with USBR Projects. Ex. Helena Valley Irrigation District, Bitterroot Irrigation District, Daly Ditches Irrigation District

Water Users Associations

Associated with State Water projects. Ex. Broadwater-Missouri Water Users Assn, Deadmans Basin Water Users Assn

Ditch or Canal Companies

Privately held. Ex. Dearborn Canal and Water Co.



Who are you guys?





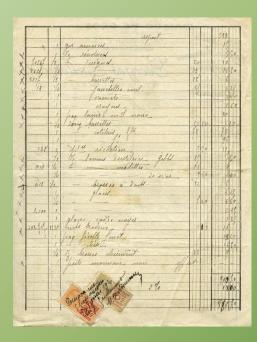




Key Players District Court Montana Water Judge Court - issues order - decree issuance Clerk of Decreed Water Users **District Court** - processes appointment **DNRC** Water - technical assistance Commissioner - training - on-the-ground

District Court Decree

- Includes all water rights dated before Decree Issued
- Does not reflect newer water rights, permits, or changes



vs. Water Court Decree

- Includes all water rights, permits, changes in appropriation, and is updated annually.

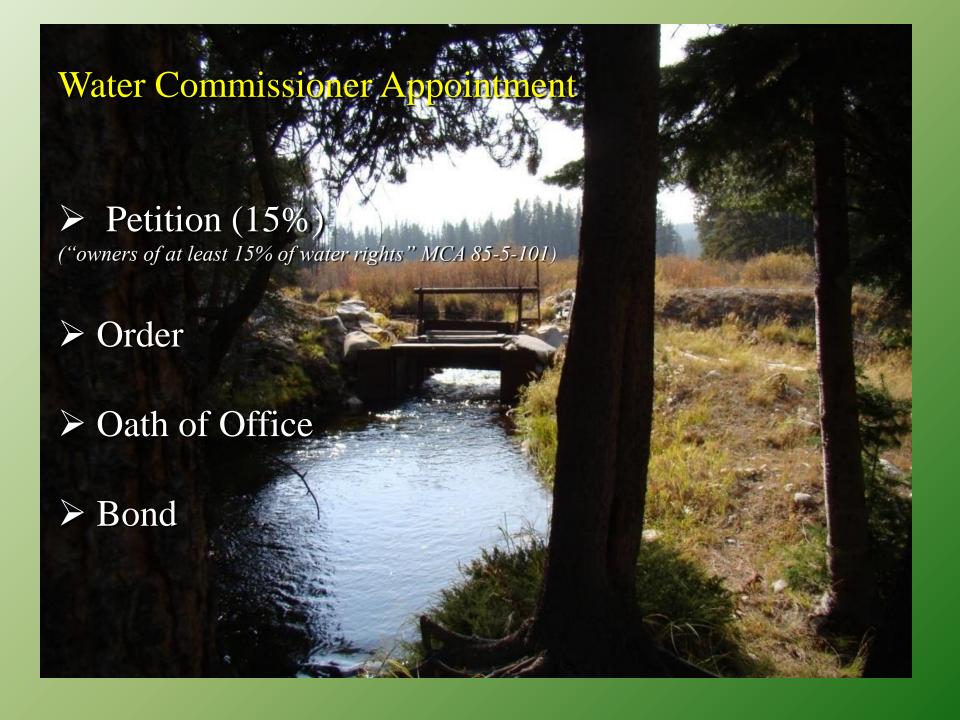
nforceable iority Date	Water Right #	Owner	Type	Use .	Acres	Pod ID	Means	Qtr Sec	Section	Twp on Rge	Source	Enf#	Diversion Name	From - To	Cfs	F
800601	43A W 11572 00	PORCUPINE CREEK RANCH INC	USE	ST		1	LS	N2SW	34	5N9E	SHIELDS RIVER	LS010	LS010	01 01 12 31		- {
830415	43A W 137659 00	MONTANA, STATE OF BOARD OF LAND COMMISSIONERS	USE	ST		1	DT	NWSENE	25	5N9E	SHIELDS RIVER	018	BECKER DITCH	01 01 12 31		
1830425	43A W 193075 00	BRIGHT, GORDON L	DECR	IR :	30.8	1	HG	SENWSW	9	4N9E	SHIELDS RIVER	012	BIG CANAL	05 01 10 04	0.43	
830425		BRIGHT, JACQUELINE J	DECR	IR :	30.8	1	HG	SENWSW	9	4N9E	SHIELDS RIVER	012	BIG CANAL	05 01 10 04		
830425	43A W 31162 00	ADAMS, DIRK S	DECR	IR 1	104	1*	HG		4	4N9E	SHIELDS RIVER	012	BIG CANAL	05 15 10 19	3.33	
330425		ADAMS, DIRK S	DECR	IR 1	104	2*	HG	NWNWNW	3	4N9E	SHIELDS RIVER	014P	ADAMS PUMP SITE	05 15 10 19		
830425	43A W 33140 00	ADAMS, DIRK S	DECR	ST		1	LS	SESW	16	4N9E	SHIELDS RIVER	LS006	LS006	01 01 12 31		
330610	43A W 113381 00	ADAMS, ANITA L	DECR	IR 2	212	1"	HG	SWSWSE	4	4N9E	SHIELDS RIVER	011	UPPER SWANDAL DITCH	04 15 10 31	1.69	
830610		ADAMS, ANITA L	DECR	IR :	212	2*	HG	SESENW	9	4N9E	SHIELDS RIVER	010	MIDDLE SWANDAL DITCH	04 15 10 31		
830610		ADAMS, ANITA L	DECR	IR 2	212	3*	HG	SENWSE	9	4N9E	SHIELDS RIVER	009	LOWER SWANDAL DITCH	04 15 10 31		
330610		ADAMS, DIRK S	DECR	IR 2	212	1*	HG	SWSWSE	4	4N9E	SHIELDS RIVER	011	UPPER SWANDAL DITCH	04 15 10 31		
830610		ADAMS, DIRK S	DECR	IR 2	212	2*	HG	SESENW	9	4N9E	SHIELDS RIVER	010	MIDDLE SWANDAL DITCH	04 15 10 31		
830610		ADAMS, DIRK S	DECR	IR 2	212	3*	HG	SENWSE	9	4N9E	SHIELDS RIVER	009	LOWER SWANDAL DITCH	04 15 10 31		
830610	43A W 11582 00	PORCUPINE CREEK RANCH INC	DECR	IR 4	425	1	HG	NWSENE	25	5N9E	SHIELDS RIVER	018	BECKER DITCH	05 15 09 19	0.56	
830610	43A W 191857 00	ADAMS, ANITA L	USE	ST		1"	DT	SWSWSE	4	4N9E	SHIELDS RIVER	011	UPPER SWANDAL DITCH	01 01 12 31		
330610		ADAMS, ANITA L	USE	ST		2*	DT	SESENW	9	4N9E	SHIELDS RIVER	010	MIDDLE SWANDAL DITCH	01 01 12 31		
330610		ADAMS, ANITA L	USE	ST		3*	DT	SENWSE	9	4N9E	SHIELDS RIVER	009	LOWER SWANDAL DITCH	01 01 12 31		

How is a decree enforced?

Through the appointment of a water commissioner who:



- ➤ Enforces priority date limits for water use.
- Monitors headgates and measuring devices to ensure compliance with decree.
- ➤ Keeps records of water use and submits reports to District Court.



Once Appointed, now what??

- > duties
- > list of water users, map, DNRC Tabulation
- > notification
- > payment system
- > worker's compensation



Water Commissioner Duties

➤ Measure and distribute waters based on Prior Appropriations Doctrine

"first in time, first in right.."





- ➤ Inspect headgates and measuring devices to ensure proper functioning condition.
- Record (daily) water distribution and expenses.
- ➤ Not apportion or distribute water if the water user fails to have a proper measuring device or fails to pay.

Water Commissioners **Do Not** Have the Following Responsibilities

- ➤ Approval of changes in points of diversion, flow rate, place of use, priority date, or period of use.
- Exemption from 310 permit for headgate and diversion dam repair.

 (Natural Streambed and Land Preservation Act)

More Things Water Commissioners Can/Should and Can't/Shouldn't Do:

<u>Yes</u>

- Access any headgate related to the POD of a water right in the decree.

 Establish with water user best way to access headgate and measuring device.
- Establish clear understanding/communication with water users pertaining to turn on/shut off. For example, there may be a lag when delivering stored water, etc.
- ➤ Work with water user if headgate or measuring device requires repair/replacement.
- > Provide clear contact info for water users (and vise-versa)

<u>No</u>

- > Trade water outside of flow rate, period of use, place of use
- ➤ Distribute water to a non-water right holder
- ➤ Distribute by purpose of use. All beneficial uses have equal standing (irrigation, stock, municipal, instream flow for fisheries, storage, etc.)

Daily Record of

Water Distribution

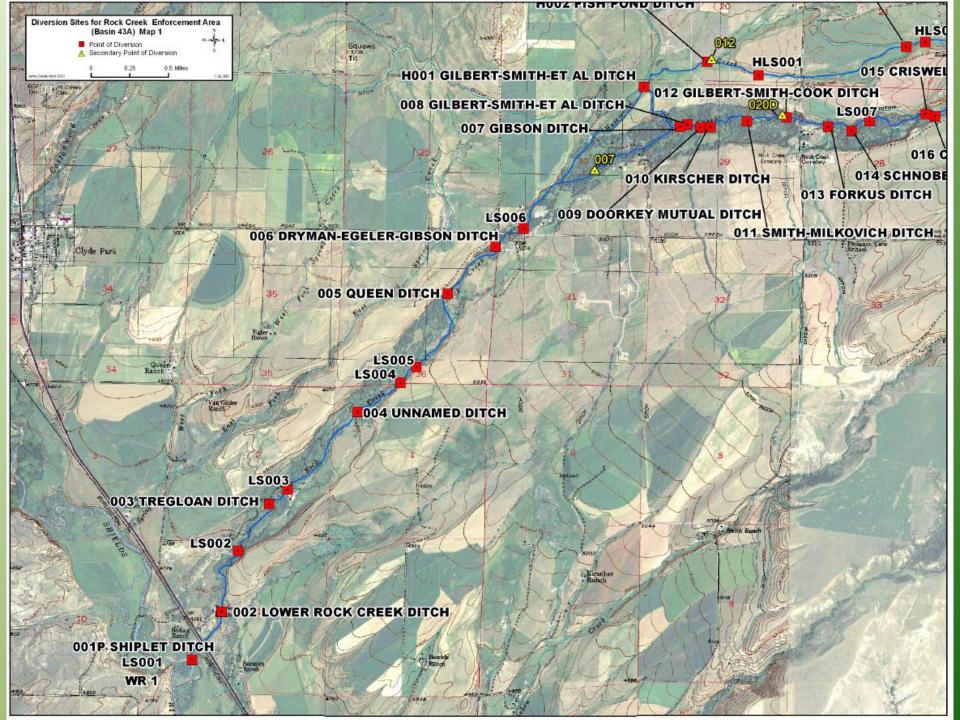
Daily allotment (inches)

Payment (wage and mileage)

MONTANA FIFTH JUDICIAL DISTRICT COURT, BEAVERHEAD COUNTY REPORT OF WATER COMMISSIONER from MAY 17-06 to JULY 19-0610 MILES mones inches inches inches inches inches inches inches inches inches PAGERWORK DAY - UAY CHARGE + MILES TO DI 336 484 199 484 220 320 320 320 180 TOTAL Commissioner expenses: SUBMITTED this 28 day of TULY, 2006

DAYS 215780 WITH Water C

Water Commissioner		MRoberts											
Stream		Spring Creek											
Year		2016											
Flow in cfs													
			1-	May	2-N	1 ay	3-N	May	4- N	May			
<u>Diversion</u>		<u>Device</u>	<u>Stage</u>	Q	<u>Stage</u>	Q	<u>Stage</u>	Q	Stage	Q			
Inflow at HWY Gage		USGS Gage		68.8		64.0		62.0		62.1			
Beck Div1	2.0	2' parshall	1.1	9.3	1.1	1.2	1.1	9.3	1	8.0			
Beck Div2	4.0	4' parshall	0.85	12.4	0.85	12.4	0.85	12.4	0.8	11.3			
Ferch Ditch	8.0	8' parshall	0.65	16.0	0.65	16.0	0.65	16.0	0.6	14.1			
Ellis Ranch	3.0	3' Contracted Rectangular Weir	0.42	2.64	0.42	2.64	0.42	2.64	0.4	2.46			
Myles Ditch	6.0	6' Contracted Rectangular Weir	0.5	6.95	0.5	6.95	0.3	3.25	0.3	3.25			
Norberg Canal	4.0	4' Cipolletti Weir	0.45	4.07	0.45	4.07	0.45	4.07	0.4	3.41			
McLane weir	6.0	6' Cipolletti Weir	0.5	7.14	0.5	7.14	0.3	3.32	0.3	3.32			
McLane Ditch	0.3	V-Notch Weir	0.4	0.26	0.4	0.26	0.4	0.26	0.3	0.13			
FWP Instream Flow Lease				10.00		10.00		10.00		10.00			
Sum of diversions				58.73		50.65		51.21		45.90			
Outflow				10.07		13.35		10.79		16.20			



\$\$\$ Payment

- Each water user pays for water they received (proportionately).
- > Payment system (MCA 85-5-204, 2007) Can receive money up front.
- ➤ Water Commissioner is paid directly through Clerk of Courts office
- ➤ If user does not pay, water can be shut off and/or court can summon to "show cause" hearing.





Expenses billed to water users include:

- > set fee billed proportionately based on water used
- > mileage
- > telephone
- repairs to headgates, ditches, measuring devices
- ➤ Commissioner Training (?)
- ➤ Worker's Compensation

(document everything)





5 South Last Chance Culch • P.O. Box 4759 • Helena, MT 59604-4759

Customer Service: 1-800-882-6102 or 406-444-6500

Praud Hotting: 1-888-582-7488 (888-MT-CRIMF)

Laurence Hubbard, President/CEO

WORKERS COM ARRANGEMENT FOR WATER COMMISSIONERS 04/1/2015 *

- 1. Term: Two options:
 - a. Short term: Policy will only run for the period requested for coverage for the water commissioner. Policy will cancel and not renew & if commissioner is appointed for another period, a new application will have to be completed & submitted.
 - b. Regular 12 month term: Policy will run for 12 months with coverage for the water commissioner being only for the months given. The application needs to be specific on the time frame required for coverage on the owner of the policy. The policy will automatically renew in 12 months as long as payrolls & payments are kept up to date.
- Binding Effective date: This will be the day following the date when 3 items have been received in MSF office:
 - a. Any prior policy reconciled (payroll reports received & payment received) if applicable.
 - b. Completed application.
 - Deposit & expense constant or 1st installment.
- 3. Coverage for water commissioner. The covered period will be from no sooner than the effective date of the policy (can use a later date) to the last date the commissioner thinks he/she will need coverage. Ex: policy starts 06/01/2014 & coverage is needed from 06/01/2014 to the end of Oct. So the last day of coverage would be 10/31/2014. If the commissioner stops earlier, it is his/her responsibility to contact MSF to request the coverage stop sooner. If the coverage is needed longer again it is the responsibility of the commissioner to notify MSF PRIOR to ending coverage date for an extension.
- 4. The 2 options of policies:
 - a. Installment method:
 - This will require a payment of at least \$410.14 down (includes the expense constant) & 2 more monthly payments to pay off the premium in advance. Usually has an annual payroll reporting frequency.

Montana's insurance carrier of choice and industry leader in service.



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Customer Service: 1-800-882-6102 or 406-444-6500

Praud Hotline: 1-888-682-7468 (888-MT-CRIME)

Laurence Hubbard, President/CEO

b. Deposit method:

- This will require the payment of the expense constant plus a 20% of the estimated premium.
- ii. The payroll reporting will be semi-annually, meaning a payroll report will be sent July & Jan. They have to be filled out & returned by due date & premium due will need to be paid by due date. These payrolls will be due the end of July & the end of Jan with the premium due the following months.

The rates for the water commissioners this year will be: \$8.21/PER \$100.

The lowest wage that commissioners can elect is \$900/month for sole proprietors. The approximate premium cost would be \$472.76to bind coverage and 2 monthly installments of \$302.76Which would be prorated when coverage is removed or cancelled as stated above. Other options may be considered.

Your contacts are Rabecca Lindal at 1-800-332-6102 ext. 5260 and Karen Beddow 5112 at 1-800-332-6102. Both of these customer service specialists will be able to assist you with any questions.

*note: 2015 adjustments are bolded. Changes made on 4/1/15 by DNRC per email contact with Rabecca Lindal from Montana State Fund.

*note: 07/01/2016 Rates change any application received after 07/01/2016 will be subject to new rates & binding amount.

Montana's insurance carrier of choice and industry leader in service.

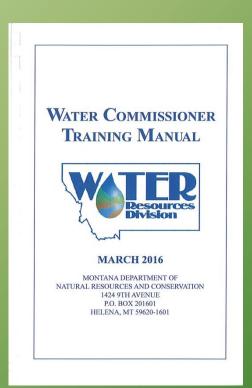


Rights and Duties of Water Users:

- Required to have suitable and functioning headgate and measuring device.
- > May file dissatisfied user complaint with judge.
- ➤ Failure of Water Commissioner to perform duty is Contempt of Court.

Special Circumstances

- water rights not in decree
- contract water
- carriage water
- temporary changes
- road construction
- instream flow/lease enforcement
- return flow
- seepage rights
- futile call





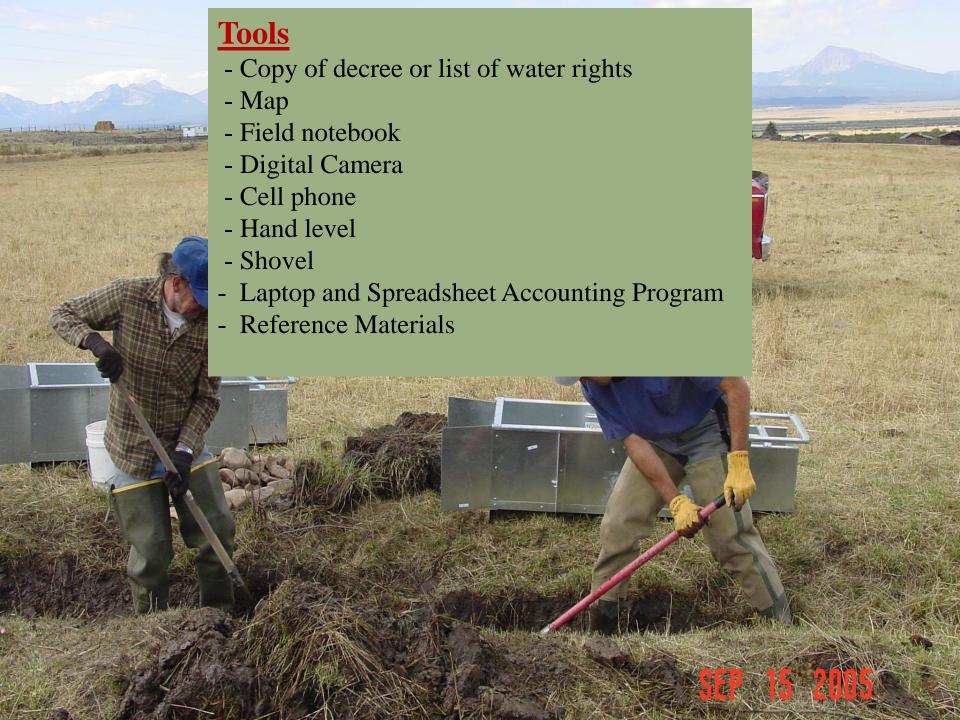
Stored Water Distribution

- > Contract water
- > Administered separate from natural flows
- ➤ May use stream channel as conveyance but must be measured at reservoir outlet (in most cases)

Document

- > date and time of anything you do
- > daily record of water distribution
- > mileage
- > telephone
- > any repairs (photo document, date)
- > repair costs
- ➤ correspondence with users, Judge, DNRC, Water Court
- > worker's compensation insurance





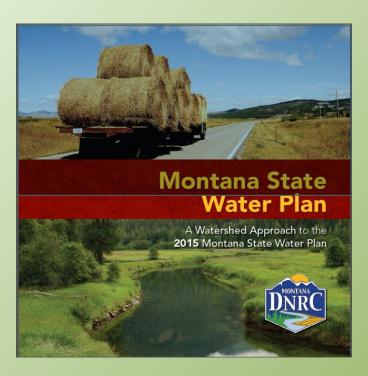
Communication

Water Commissioner and Water User Water Commissioner and District Court



Issues that require Communication include:

- ➤ Turning on/off
- ➤ Headgate Adjustment
- > Access
- > Repair/Replacement
- > Payment Issues



Water Commissioner
Training Requirement and
Certification??

SHORT TERM RECOMMENDATION (0-2 YEARS)

Continue funding of both the Water Court and the DNRC efforts to complete the current adjudication process at the necessary level of staffing to meet legislatively established benchmarks.

INTERMEDIATE TERM RECOMMENDATION (2-6 YEARS)

The DNRC and the Water Court should work with stakeholders to evaluate and develop processes to ensure water rights are accurately and consistently defined across Montana.

LONG TERM RECOMMENDATION (6 - 10 YEARS)

Create a plan for transitioning the state, including the DNRC, the Water Court, and the District Courts, to post adjudication water distribution, management and enforcement roles.

Enforce Against Illegal Water Use

Montana Water users want a more efficient, less expensive, and more administrative approach to water right enforcement. There is growing public sentiment in support of DNRC playing a more active enforcement role against illegal water use.

INTERMEDIATE TERM RECOMMENDATIONS (2-6 YEARS)

- Examine and recommend changes to improve the current administrative process for bringing enforcement action against illegal water use.
- DNRC and the Water Court should create and the Legislature should actively fund a water rights dispute mediation unit to provide an administrative alternative to traditional water rights litigation. Training in dispute resolution and mediation should be available to all water commissioners.
- Review the procedures for establishing water distribution projects based upon Water Court decrees to improve the efficiency of that process.

Promote consistent legal and professional measurement and distribution of water under decree by requiring water commissioners to complete the DNRC training (MCA 85-5-111) and create a certification process with annual renewals.

LONG TERM RECOMMENDATION (6-10 YEARS)

Clarify how decrees within subbasins of major adjudicated basins will be administered when a water rights dispute arises between water users in adjacent subbasins.

Provide Sufficient Information, and Legal and Administrative Capacity to Minimize Adverse Effects during Times of Water Scarcity

Climate variation and shifting weather patterns affect average temperatures, the amount, and distribution of precipitation, and whether that precipitation occurs as rain or snow. As a result, seasonal streamflows are likely to change both in volume and timing. Climate variation may lead to an extended growing season and/or higher water use by crops and vegetation. Looking ahead, our water management strategies must adapt to address the highly variable water supply.

INTERMEDIATE TERM RECOMMENDATIONS (2-6 YEARS)

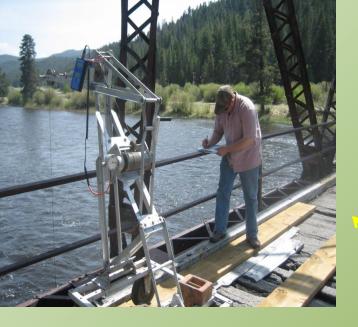
- Drought planning efforts must include legal and administrative mechanisms that enable water users to reduce water use without the risk of abandonment and allow for the water savings to be protected.
- Assess the water right implications and potential adverse effects of allowing a water right holder to change their period of use to adapt to changing runoff and growing seasons.

Analyze Additional Opportunities and Challenges for Using Water Marketing, Mitigation, and Banking as Tools for Meeting New Demands

Compared to many western states. Montana appears to have relatively abundant water supplies, however most of this water may already be appropriated, and many parts of the state are fully allocated and closed to new appropriations. Meeting new water demands requires innovative approaches to address local water deficits within individual sub-basins. Understanding the potential positive and negative impacts of these measures is the first step towards taking advantage of new approaches. The potential for water marketing (the sale of water or the water right by the owner) is high in Montana, especially in closed basins where the value of water increases with new water demands. Mitigation for new uses will require the reallocation of surface water or groundwater through a water right change. There are questions about the scope of water banking and its role in facilitating the reallocation of water, and the potential adverse effects of change authorizations. These issues and opportunities for mitigation, water marketing and water banking require more research. innovation, and application in the next

INTERMEDIATE TERM RECOMMENDATIONS (2-6 YEARS)

- Assess the opportunities, challenges, water right implications, and potential adverse effects of using water marketing, mitigation, and banking as tools for meeting new demands
- Create well-managed systems that offer economic development opportunities to market, transfer and lease water and build public awareness of water marketing opportunities.





Take Precautions







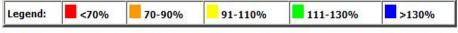




MONTANA SNOTEL Snow Water Equivalent Update Graph

As of WEDNESDAY: APRIL 6, 2016

Basin	Snow Water Equivalent Percent of Median						
KOOTENAI RIVER BASIN	98%						
FLATHEAD RIVER BASIN	97%						
UPPER CLARK FORK RIVER BASIN	95%						
BITTERROOT RIVER BASIN	96%						
LOWER CLARK FORK RIVER BASIN	88%						
JEFFERSON RIVER BASIN	107%						
MADISON RIVER BASIN	98%						
GALLATIN RIVER BASIN	99%						
MISSOURI HEADWATERS	102%						
HEADWATERS MISSOURI MAINSTEM	95%						
SMITH, JUDITH, AND MUSSELSHELL RIVER BASINS	108%						
SUN, TETON AND MARIAS RIVER BASINS	53%						
MISSOURI MAINSTEM RIVER BASIN	90%						
ST MARY AND MILK RIVER BASINS	80%						
UPPER YELLOWSTONE RIVER BASIN	93%						
WIND RIVER BASIN (WYOMING)	102%						
SHOSHONE RIVER BASIN (WYOMING)	94%						
BIGHORN RIVER BASIN (WYOMING)	90%						
TONGUE RIVER BASIN (WYOMING)	74%						
POWDER RIVER BASIN (WYOMING)	90%						
LOWER YELLOWSTONE RIVER BASIN	92%						



^{* =} Data are not available or data may not provide a valid measure of conditions for over half of the sites within the basin.

MONTANA SNOTEL Snow Water Equivalent Update Graph

As of MONDAY: APRIL 6, 2015

Basin	Snow Water Equivalent Percent of Median
KOOTENAI RIVER BASIN	54%
FLATHEAD RIVER BASIN	82%
UPPER CLARK FORK RIVER BASIN	87%
BITTERROOT RIVER BASIN	80%
LOWER CLARK FORK RIVER BASIN	56%
JEFFERSON RIVER BASIN	75%
MADISON RIVER BASIN	69%
GALLATIN RIVER BASIN	85%
MISSOURI HEADWATERS	75%
HEADWATERS MISSOURI MAINSTEM	78%
SMITH, JUDITH, AND MUSSELSHELL RIVER BASINS	82%
SUN, TETON AND MARIAS RIVER BASINS	64%
MISSOURI MAINSTEM RIVER BASIN	74%
ST MARY AND MILK RIVER BASINS	62%
UPPER YELLOWSTONE RIVER BASIN	83%
WIND RIVER BASIN (WYOMING)	69%
SHOSHONE RIVER BASIN (WYOMING)	75%
BIGHORN RIVER BASIN (WYOMING)	78%
TONGUE RIVER BASIN (WYOMING)	73%
POWDER RIVER BASIN (WYOMING)	77%
LOWER YELLOWSTONE RIVER BASIN	73%

Legend: 70% 70-90% 91-110% 111-130% >130%

^{* =} Data are not available or data may not provide a valid measure of conditions for over half of the sites within the basin.

Montana DNRC State Water Projects Bureau Reservoirs



MONTANA DEPARTMENT OF NATURAL RESOURCES AND CONSERVATION

WATER RESOURCES DIVISION - STATE WATER PROJECTS BUREAU

March 31, 2016

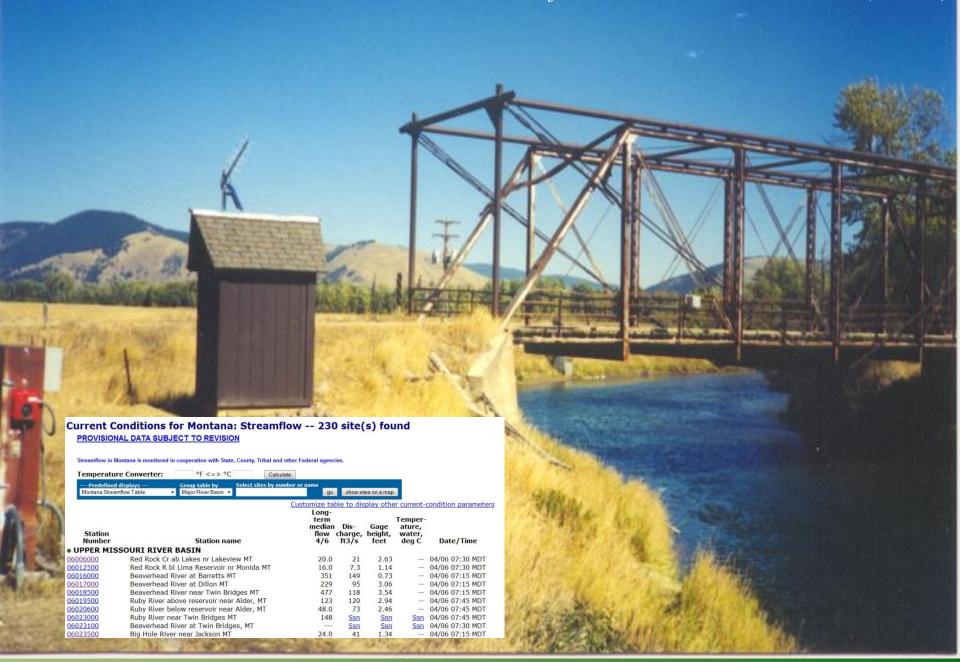
All Contents in Acre-Feet TOTAL CAPACITY RESERVOIR CONTENTS (includes dead storage)* AVERAGE Last Year PRESENT % CAPACITY %AVERAGE READING Full Pool **Last Month** COMMENTS 1960 - 2014 3/31/2015 2/29/2016 3/31/2016 3/31/2016 3/31/2016 DATE Contents ACKLEY 4/2/2016 elev.= 4307.2 6.722 3,231 3,997 3,942 4,194 62 130 BAIR 7,300 4.373 5,609 5,668 78 130 4/1/2016 elev.=5318.5 5,148 COONEY 28,230 20.912 22,280 24,440 23,480 83 112 3/22/2016 elev.=4245.4 (23,390 AF) COTTONWOOD 1.900 1.014 1.940 1.284 1.805 95 178 4/1/2016 elev.= 5102.0 DEADMAN'S BASIN 75.968 49.256 70,577 58,923 63,427 83 129 3/31/2016 | elev.=3914.7 (59,677 AF) E.F. ROCK CREEK 16.040 9.591 11.045 8,142 8.751 55 91 3/31/2016 elev.=6034.5 3/31/2015 No Data Reported FRENCHMAN 2.777 2,156 2,777 2.777 2,777 100 129 MARTINSDALE 23,348 9.135 19,337 14,918 15,648 67 171 3/31/2016 elev.=4770.5 MIDDLE CREEK 10.184 6,163 5,818 5,551 5,557 55 3/31/2016 elev =6697.6 58 NEVADA CREEK 11,207 7,819 10,861 5,116 6,496 83 4/3/2016 elev.=4601.7 75 NILAN 10,992 6,707 10,020 6,315 8,207 122 4/5/2016 elev.=5477.5 N.FK. SMITH RIVER 11,406 7.082 10,330 6.383 6,789 60 96 3/31/2016 volume estimated RUBY RIVER 87 3/31/2016 volume estimated 37.612 31,222 37,137 29,124 32,900 105 TONGUE RIVER 79 071 50 139 56,093 54,262 58,295 74 116 3/31/2016 elev.=3422.0 (57.584 AF) W.F. BITTERROOT 32,362 9.221 20,019 8.775 16,126 50 175 3/29/2016 elev.=4692.65 (15.470 AF) WILLOW CREEK 18,000 16,386 16,127 15.010 15,560 86 95 3/31/2016 volume estimated 1,250 3/31/2016 elev = 3116.7 YELLOWATER 3.842 3.236 3,160 3.171 83 254 * Note: Reservoir contents include dead storage at the following: 1001 AF 28 O&M slope storage table does not include dead storage (so dead storage has to be added into the storage from the table) Ackley Cooney 90 AF Tonque River 711 AF (O&M storage table includes dead storage) 3750 AF ** Deadman's W. F. Bitterroot 656 AF (O&M storage table includes dead storage) 900 AF Willow Creek 269 AF (O&M storage table includes dead storage) * Note: Cooney capacity reflects capacity after 1982 dam rehabilitation; prior capacity was 24,195 A.F.. Average storage shown is for post rehabilitation data. * Note: Middle Creek capacity reflects capacity after 1993 dam rehabilitation; prior capacity was 8,027 A.F., Average storage shown is for post rehabilitation data. * Note: Nevada Creek Reservoir Capacity reflects live storage capacity survey conducted in year 2000. Prior live storage capacity documented as 12,723 AF.

* Note: Tongue River capacity reflects capacity after 1999 dam rehabilitation; prior capacity was 68,040 A.F.. Average storage is post rehabilitation data.

* Note: Frenchman Reservoir capacity tables updated based on aerial survey; prior capacity was 3752 A.F. Average shown is pre aerial survey.



Blackfoot River abv Nevada Creek (USGS)



MUSSELSHELL RIVER DISTRIBUTION PROJECT



Photo courbey of Big Sty Finding.com

The Musselshell River Distribution Project involves the administration of decreed water on over 200 miles of the Musselshell River, from the North Fork and the South Fork to the USGS gage station at Mosby, and all waters considered by the Montana Water Court to be a part of the lower Musselshell River below the U.S.G.S. gauge station at Mosby, Montana. This page provides information for water users within the project boundaries. Water users can view the priority dates entitled to receive water, and can place orders for water directly with the Deputy Assistant Vater Commissioner (John Rouane) by filling out the form below. This page also provides links to agencies and information of interest to water users.

Your chief water commissioner will be posting news updates. Please click on the icon below to take you to the page where you can read the latest news!

News last update 8/24/2015



Priority Date

Zone 4	Zone 2A	Zone 2B	Zone 3	Zone 4	Zone 5	-Zine 6
7-1-1985	10-1-1903	10-1-1903	11-14-1891	05-10-1938	7-31-1882	4-10-1885

Effective 9/16/2015 (See News Updates)

Water Order Form for Existing Water Rights

> 2015 Irrigation Season (You must fill out ALL fields on this form!)

Important Contact Info & Links

Chief Water Commissioner Peter Marchi ester@minouch.net 406-572-3410 or 3307 406-220-1947

Water Commissioners

Leon Hammond ammond@listriangle.com 406-220-2093

Bonnie Stensvad stens v ad@wildblue.net 406-358-2235 406-320-2454

John Rouane John,rouane@live.com 406-281-1758 (primary)

Deadman's Basin Project Manager Teri Hice kthice@midrivers.com

> OLICK HERE DBWUA WEBSITE



Nevada Creek - Current Info

- Current Flow above reservoir
- · Current Reservoir Measurements
- Current Flow below reservoir
 Current Status of Israelion Dia
- Current Status of Irrigation Rights
 Upper Nevada creek drainage

Nevada Creek W.U.A.

· Historical Irrigation Statistics

Irrigation Rights Listing "Nevada Creek Drainage"

- Flow Chart Nevada creek drainage
- Drainage section designations
- ↓ listings: sorted by priority date ↓

 Upper Nevada creek drainage rights Sections A-ND-B
- Sections A-ND-B

 Spring creek area rights
 Section S
- Cottonwood creek drainage rights Section C
- Section C
 Douglas creek drainage rights
 Section D
- McElwain creek drainage rights
 Section M

March 2016 Water Supply Outlook Report

March 8th, 2016

The NRCS has released their Water Supply Outlook Report for March. As always, you can view / download the latest full state-wide report here, or also go to the Snowpack & Precioitation Reports page.

There is some reduction in this current forecast vs last months 98% of normal. Nevada creek is now forecast at 92% of normal flow for both the April-May & April-July time periods (at the most probable 50% chance of exceedance). These predictions are made with the assumption of normal precipitation over the remaining late winter – spring.

Considering this forecast flow volume, it is still quite possible that we achieve full capacity for Nevada creek reservoir. A lot will depend on how the runoff comes. An early, fast runoff would result in a large percentage of that volume to be stored. A slower, extended runoff could result with much of the volume being consumed as senior irrigation rights would have to be honored. And as always, the addition of heavier precipitation can change things quick-

(click chart image to enlarge)

Upper Clark Fork River Basin Streamflow Forecasts - March 1, 2016

Forecast Exceedance Probabilities for Risk Assessme

Recently Most Viewed

Current Reservoir Measurements

Vater Rights Query System (Montan D.N.R.C)

 Snowpack & Precipitation Reports – Stream

Water Supply Information

- . Snowpack & Precipitation Reports
- Snotel Sites Nevada Ck area
- Streamflow Gage Sites

More Water References

- Water Rights Query System (MT-DNRC)
- Montana Cadastral land ownership
 lookup
- Powell County Water Resources Survery (1959)
- Basic Montana Water Law
- Water Rights in Montana Handbook

Water Measurement Manual - USBR

Subscribe to Email Updates

Water Management

- State Water Plan
- Regional River Basin Information
- Programs, Projects, and Studies
- Reports & Technical Information
- Training & Education
 - Water Commissioner Information
 - Water Commissioner Training Materials
- Transboundary Water Information

Water Compact Information

- St. Mary Rehabilitation Project
- Water Operations
- Water Projects
- Water Rights Bureau

Water Events

Events

Manage portlets

Water Commissioner Information

by cn2157 — last modified Apr 01, 2016 12:35 PM — History

DNRC has developed an educational program for water commissioners that provides annual training on commissioner responsibilities and water measurement techniques. This free training is available to anyone interested in the accurate measurement and distribution of water in Montana including state and federal water managers, water commissioners, ditch riders and dam tenders, District Court and Water Court personnel.

Training Materials



Decree Status



FAOS



Stream Flow Data



District Court Information

Forecasting





Water Commissioner Law



Active Water Commissioners



Glossarv



Educational Materials

Water Rights and Historical Beneficial Use

- · Water Rights and Historical Beneficial
- · Water Rights and Historical Beneficial Use (Video)

Facts About Montana's Water

- 1. Who Owns the Water in Montana?
- 2. Who Manages Montana's Water?
- 3. How Much Water is There and How is it Used?
- 4. Can Groundwater Meet the Demand for New Water Uses in Montana?
- 5. What is the History of Water Planning in
- 6. How is Water Managed in the Event of
- Water Shortages? 7. What is Water Rights Adjudication?
- 8. How Are Instream Flows Protected?
- 9. What Are Federal Reserved Water Rights?
- 10. Water Fact Sheets Terminology

Guide to Montana Water Management

Who Does What with Montana's Water Resources?

Manage portlets



Divisions

Board of Oil & Gas Conservation and Resource Development Director's Office Forestry Reserved Water Rights Compact Commission

About

DNRC's Mission Administrative Rules Organizational Structure Boards and

Follow Us



Useful Websites and Contacts

Montana Department of Natural Resources and Conservation (DNRC) http://dnrc.mt.gov/divisions/water

- Water Right Forms and Records, http://dnrc.mt.gov/divisions/water/water-rights
- Adjudication http://dnrc.mt.gov/divisions/water/adjudication
- Reservoir Operations
 http://dnrc.mt.gov/divisions/water/projects
- Water Commissioner Course Information (manual, power point, etc.)
 http://dnrc.mt.gov/divisions/water/management/watershed-planning

DNRC Water Resources Regional Offices

Billings: (406) 247-4415 Bozeman: (406) 586-3136 Glasgow: (406) 228-2561 Havre: (406) 265-5516 Helena: (406) 444-6999 Kalispell: (406) 752-2288 Lewistown: (406) 538-7459 Missoula: (406) 721-4284

Current Streamflow Conditions - United States Geological Survey (USGS)

http://waterdata.usgs.gov/mt/nwis/current/?type=flow

Current Snowpack Conditions - Natural Resources and Conservation Services (NRCS)

http://www.nrcs.usda.gov/wps/portal/nrcs/detail/mt/snow/?cid=nrcs144p2 057794

Web Soil Survey - NRCS

http://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm

Current Drought and Water Supply Conditions - State of Montana

http://drought.mt.gov/default.aspx

Groundwater/Well Information Montana Bureau of Mines and Geology (MBMG)

http://www.mbmg.mtech.edu/grw/grw-main.asp

DNRC REGIONAL OFFICES



Area 1

Kalispell Regional Office 655 Timberwolf Plovy, Suite-4 Kalispell, MT 59901 (400) 752-2288 Area 2

Hwite Regional Office 210 Soth Avenue Heim, MT 59501 (406) 265-5516 Area 3

Glasgow Regional Office 222 Sudh Street South Glasgow MT 59230 406(228-256) Area 4

Missoula Regional Office 2705 Spurgin Road Building C Missoula, MT 59806 (406) 721-4284 Area 5

Helena Regional Office 1424 Ninth Avenue Helena, MT 59620 [406] 444-6999 Area 6

Lewistown/Regional Office 613 NE Main, Suite E Lewistown, MT 59457 (406) 538-7459 Area 7

Bozernan Regional Office 2273 Boot Hill Court Suite 110 Bozernan, MT 59715 (400) 586-3136 Area 8

Billings Regional Office Airport Business Park 1371 Pirmtop Drive Billings, MT 59105 (406) 247-4415 DNRC Helena Water Resources REGIONAL OFFICE



Water Resources Division

The Helena Regional offices service the following counties:

Beaverhead

Broadwater

Deerlodge

<u>Jefferson</u>

Lewis and Clark

Powell

Silverbow

Deputy Regional Manager – Bryan Gartland	444-5783
Regional Office Engineering Specialist – John Connors	444-9724
Hydrologist/Specialist – Russell Gates	444-6602
Compliance Technician – Kristeen Wofford	444-6999
Water Resource Specialist – Jennifer Daly	444-6862
Water Resource Specialist – Terry Scow	444-6753
Water Resource Specialist – Myles VanHemelryck	444-6810



STREAM GAGE PROGRAM



Montana Department of Natural Resources and Conservation Water Management Bureau



Stream Gage Installation Examples











Stream Gage Installation Examples



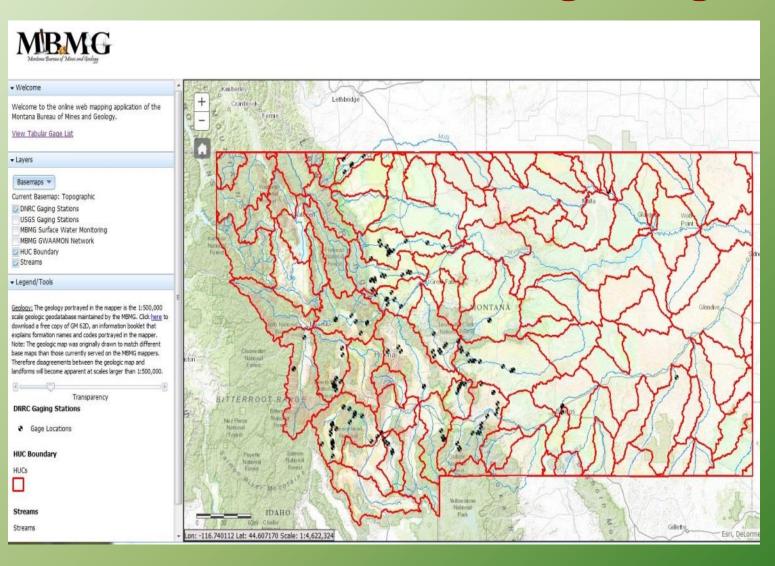


Stream Gage Installation Examples



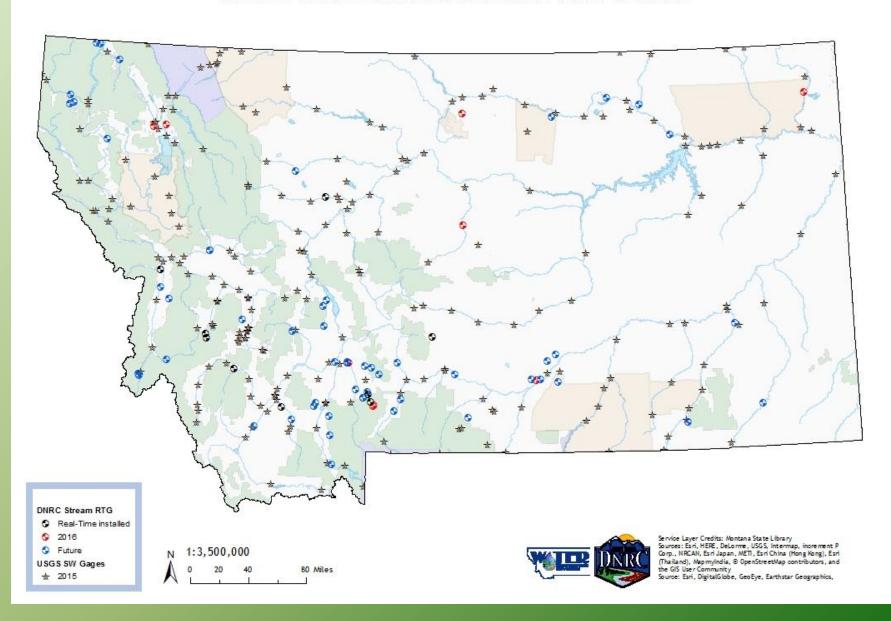


DNRC-Stream Gage Program



- Real-time stream flow information
- Water
 Temperature
 Data
- Legacy/Seasona
 1 Data
- Interactive Map will allow easy access to
 streamflow data
- Data available for viewing or download
- Statistics,
 Graphs, and
 Reports
 available to
 public

STATEWIDE DNRC REAL-TIME STREAM GAGE NETWORK





Questions?

- 1) List in order, coldest to hottest: Thompson Falls, Helena, Wisdom, Hamilton
- 2) According to MapQuest, which town is the furthest from Alzada, MT? Kansas City, MO, Yaak, MT, or Dumas, TX
- 3) What river was made famous in the Norman Maclean book,
- "A River Runs Through It"?
- 4) Name a famous actor or musician who grew up in Helena?
- 5) How many Cottonwood Creeks in Montana?
- 6) In 1999, residents of Saco, MT set a world record by cooking a 6000 pound hamburger. How many cows did that require?

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Myrna Loy



Gary Cooper



Colin Meloy

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Myrna Loy



Gary Cooper



Colin Meloy

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Myrna Loy



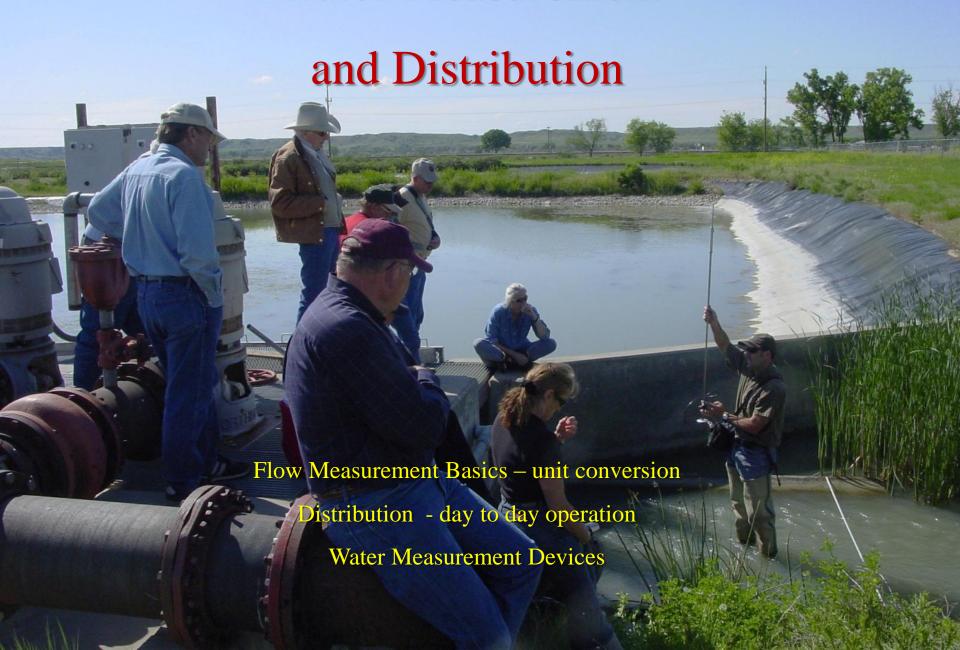
Gary Cooper



Colin Meloy

- 5) How many Cottonwood Creeks in Montana? 71
- 6) In 1999, residents of Saco, MT set a world record by cooking a 6000 pound hamburger. How many cows did that require? 17

Water Measurement



 Flow Rate or discharge is the volume of water passing a flow section per unit time

Standard units of cubic feet per second (cfs)

- Basic flow equation
 - Flow Rate (discharge) = Area · Velocity

$$ightharpoonup Q = A \cdot V$$

$$30 ft^2 \bullet 3 \frac{ft}{\text{sec}} = ?$$

- Basic flow equation
 - Flow Rate (discharge) = Area · Velocity

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$$30 ft^2 \bullet 3 \frac{ft}{\text{sec}} = ?$$

- Flow rate (discharge) units
 - The standard unit for flow rate or discharge is cubic feet per second (cfs)

1 cfs is equivalent to:

- Flow rate (discharge) units
 - The standard unit for flow rate or discharge is cubic feet per second (cfs)

1 cfs is equivalent to: 40 miner's inches in Montana

448.8 gallons per minute

(gpm)

1.98 ac-ft per day

- Flow rate (discharge) units continued
 - A commonly used unit for flow rate or discharge is miner's inches or "inches"

1 Miner's inch in Montana is equivalent to:

1/40 cfs

11.22 gpm

1/20 ac-ft per day

Volume Units

- Standard unit of volume is acre-feet (ac-ft)
- An ac-ft is equivalent to a foot of water on one acre.

1 ac-ft is equivalent to: 325,851 gallons

43,560 cubic feet

Example Problems



WCT Manual: inside cover or page 55

- 1) What is their water right in cubic feet per second (cfs)?
- 2) Convert their water right to gallons per minute (gpm).
- 3) Convert to gallons per day (gpd).
- 4) How many acre-feet (af) is the irrigator entitled to in 10 days?

```
1 cfs = 40 m.i.

1 cfs = 448.8 gpm

1 cfs for 24 hrs = 1.983 acre-feet
```

1) What is their water right in cubic feet per second (cfs)?

$$140 \text{ in.} / 40 \text{ in.} = 3.5 \text{ cfs}$$

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2) Convert their water right to gallons per minute (gpm).

$$3.5 \text{ cfs} * 448.8 \text{ gpm} = 1570.8 \text{ gpm}$$

- 3) Convert to gallons per day (gpd).
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2) Convert their water right to gallons per minute (gpm).

$$3.5 \text{ cfs} * 448.8 \text{ gpm} = 1570.8 \text{ gpm}$$

- 3) Convert to gallons per day (gpd).
 - 1570.8 gpm * 60 min/hr * 24 hr/day = 2.26 million gallons
- 4) How many acre-feet (af) is the irrigator entitled to in 10 days?

1) What is their water right in cubic feet per second (cfs)?

$$140 \text{ in.} / 40 \text{ in.} = 3.5 \text{ cfs}$$

2) Convert their water right to gallons per minute (gpm).

$$3.5 \text{ cfs} * 448.8 \text{ gpm} = 1570.8 \text{ gpm}$$

- 3) Convert to gallons per day (gpd). 1570.8 gpm * 60 min/hr * 24 hr/day = 2.26 million gallons
 - 2 · in any 2 · in any
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$$3.5 \text{ cfs} * 448.8 \text{ gpm} = 1570.8 \text{ gpm}$$

- 3) Convert to gallons per day (gpd).
 - 1570.8 gpm * 60 min/hr * 24 hr/day = 2.26 million gallons
- 4) How many acre-feet (af) is the irrigator entitled to in 10 days?

A different irrigator is entitled to 400 acre-feet over a period of 20 days. Assuming irrigation is non-stop, what is their flow rate in cfs?

400 acre feet / 20 days = 20 ac-ft/d / 1.983 = 10.1 cfs

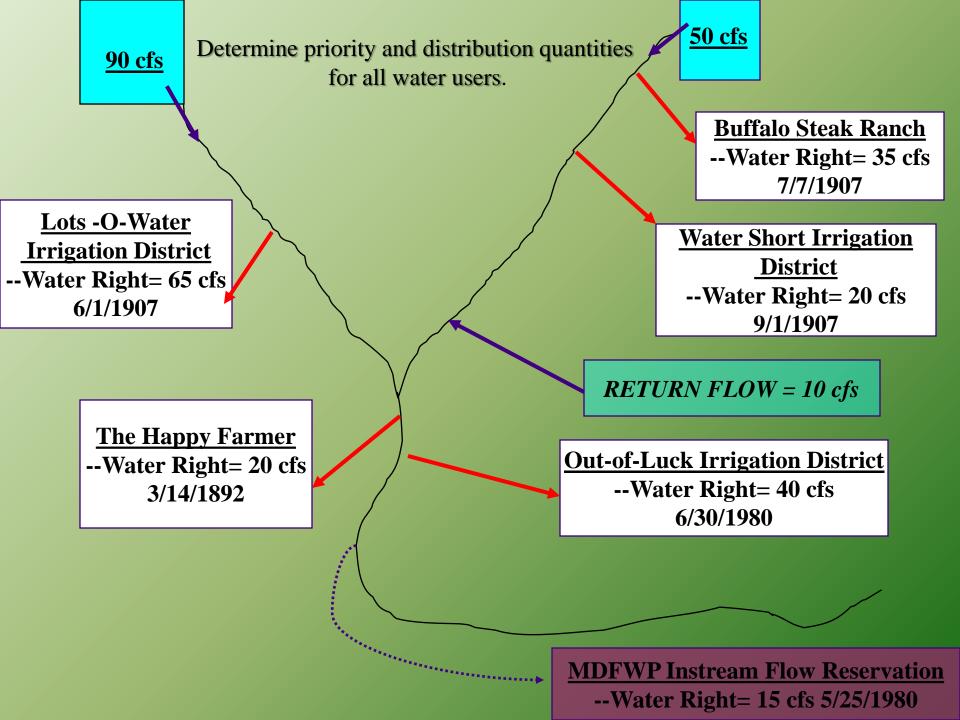
Water Distribution

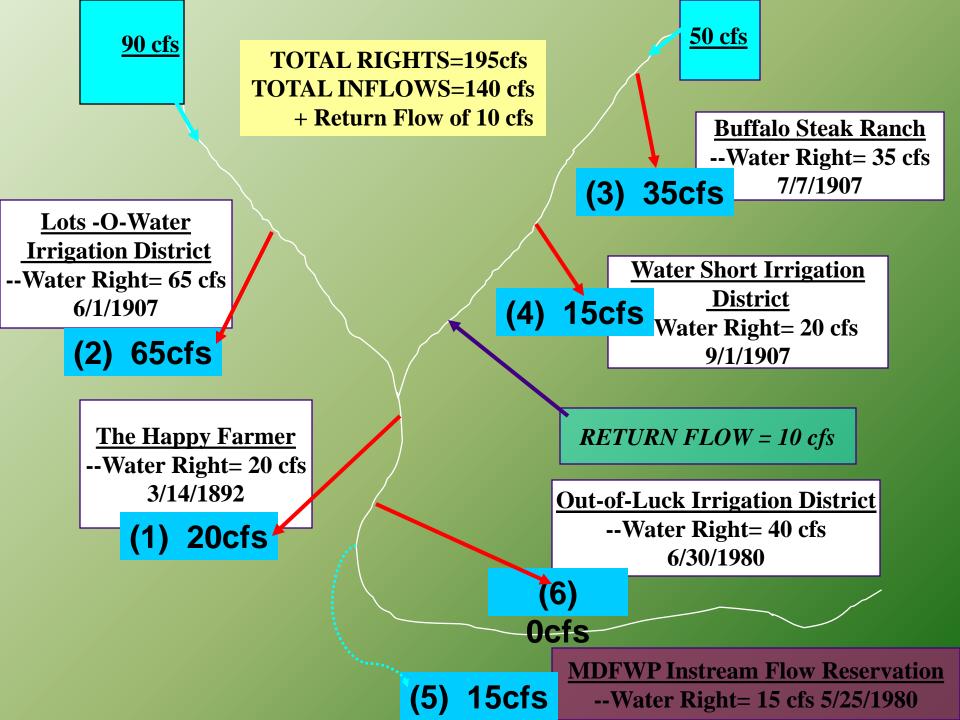
- Priority and Instream Flow
- Decreed vs. Stored Waters
- Understanding hydrology of system

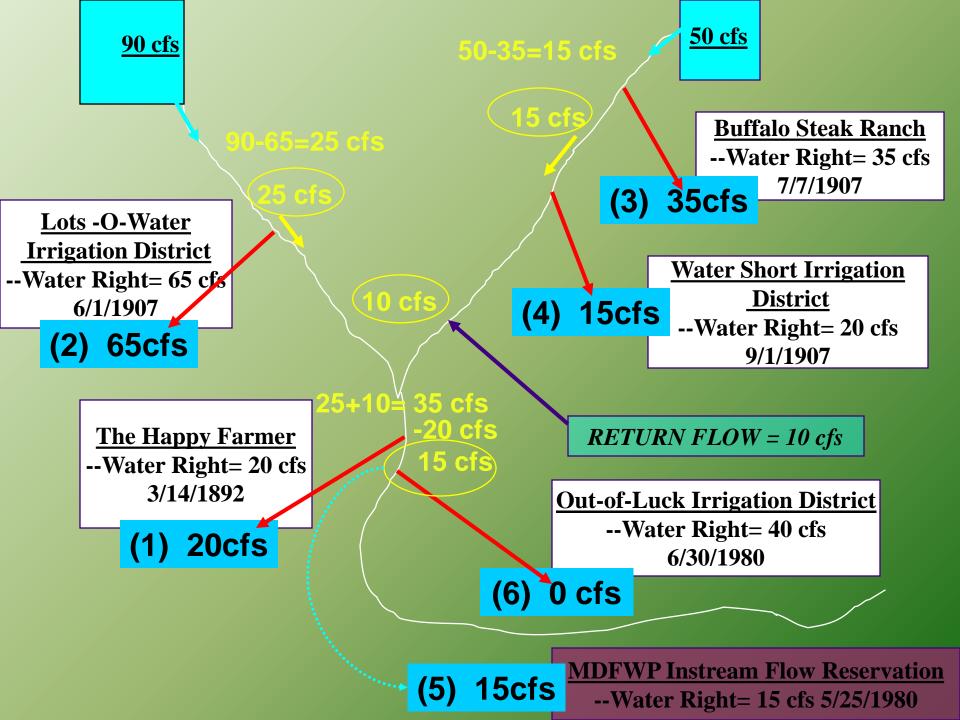


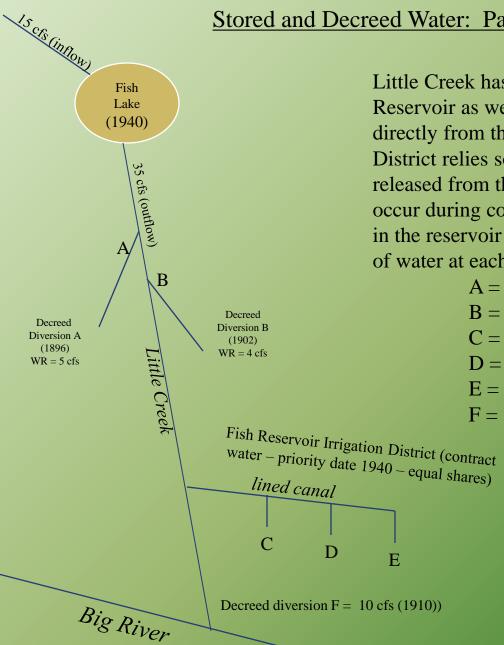




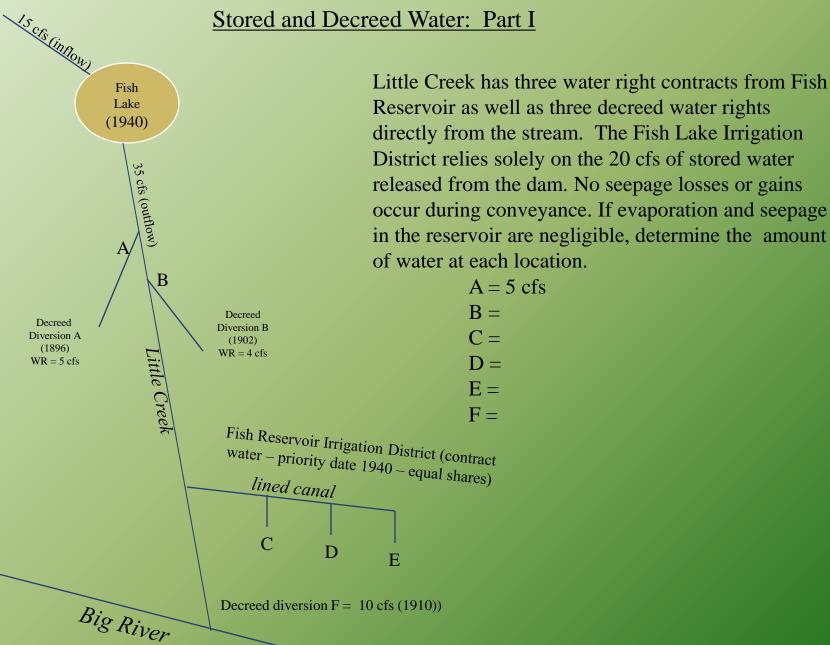








Little Creek has three water right contracts from Fish Reservoir as well as three decreed water rights directly from the stream. The Fish Lake Irrigation District relies solely on the 20 cfs of stored water released from the dam. No seepage losses or gains occur during conveyance. If evaporation and seepage in the reservoir are negligible, determine the amount of water at each location.



15 cfs (inflow)

Decreed

Diversion A

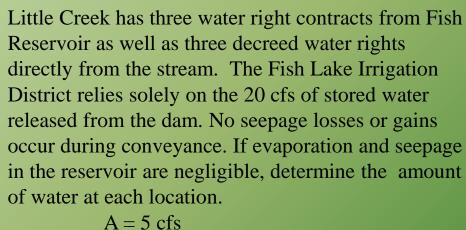
(1896)

WR = 5 cfs

Fish Lake

(1940)

Big River



$$A = 5 \text{ cfs}$$

$$B = 4 cfs$$

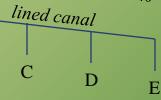
$$C =$$

$$D =$$

$$E =$$

$$F =$$

Fish Reservoir Irrigation District (contract water – priority date 1940 – equal shares)



Decreed

Diversion B

(1902)

WR = 4 cfs

Decreed diversion F = 10 cfs (1910)

15 cfs (inflow)

Decreed

Diversion A

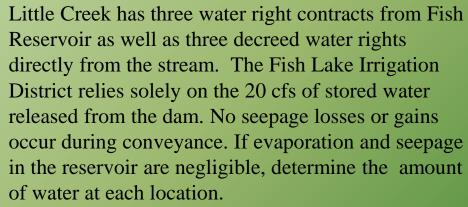
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WR = 5 cfs

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(1940)

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A = 5 cfs

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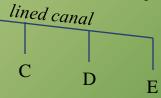
C = 6.67 cfs

D = 6.67 cfs

E = 6.67 cfs

F =

Fish Reservoir Irrigation District (contract water – priority date 1940 – equal shares)



Decreed

Diversion B

(1902)

WR = 4 cfs

Decreed diversion F = 10 cfs (1910)

Little Creek has three water right contracts from Fish Reservoir as well as three decreed water rights directly from the stream. The Fish Lake Irrigation District relies solely on the 20 cfs of stored water released from the dam. No seepage losses or gains occur during conveyance. If evaporation and seepage in the reservoir are negligible, determine the amount of water at each location.

$$A = 5 \text{ cfs}$$

$$B = 4 cfs$$

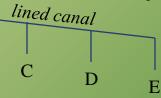
$$C = 6.67 \text{ cfs}$$

$$D = 6.67 \text{ cfs}$$

$$E = 6.67 \text{ cfs}$$

$$F = 6 cfs$$

Fish Reservoir Irrigation District (contract water – priority date 1940 – equal shares)



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15 cfs linflow)

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(1940)

15 cfs lintlow)

Decreed

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(1896)

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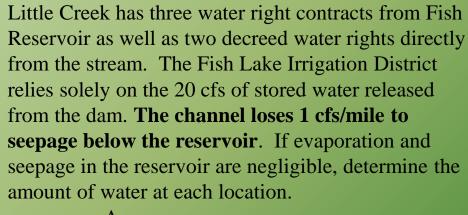
distance = 1 mile

(distance = 2 miles)

(distance = 3 miles)

Fish Lake

(1940)



A = B = C = D =

D = E =

F =

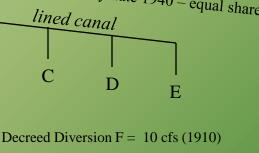
Fish Reservoir Irrigation District (contract water – priority date 1940 – equal shares)

Decreed

Diversion B

(1902)

WR = 4 cfs



15 cfs lintlow)

Decreed

Diversion A

(1896)

WR = 5 cfs

distance = 1 mile

(distance = 2 miles)

(distance = 3 miles)

Fish Lake

(1940)

Little Creek has three water right contracts from Fish Reservoir as well as two decreed water rights directly from the stream. The Fish Lake Irrigation District relies solely on the 20 cfs of stored water released from the dam. The channel loses 1 cfs/mile to seepage below the reservoir. If evaporation and seepage in the reservoir are negligible, determine the amount of water at each location.

$$A = 5 \text{ cfs}$$

$$B = 4 cfs$$

$$C = 6 cfs$$

$$D = 6 cfs$$

$$E = 6 cfs$$

$$F = 5 \text{ cfs}$$

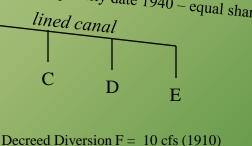
Fish Reservoir Irrigation District (contract water – priority date 1940 – equal shares)

Decreed

Diversion B

(1902)

WR = 4 cfs



A small reservoir has 25,000 acre-feet of water in storage on July 1. For the sake of this problem, assume no seepage or evaporation occurs. Between July 1 and August 31, average reservoir inflows equal 15 cfs. Irrigators require 3200 inches, 24 hours a day, from the reservoir. Lakeside residents constantly pump 2750 gpm from the reservoir for domestic water supply and water must be released from the dam at a rate of 7.5 cfs to satisfy FWP's in-stream flow lease for west-slope cutthroat. How many acre-feet of water are left in the reservoir on September 1?

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September 1 storage = (July storage + Inflows) - (Outflows)

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September 1 storage = (July 1 storage + Inflows) - (Outflows)

Inflows: 15 cfs * 1.983 acre-feet/day/cfs * 62 days = **1844 acre-feet**

Outflows: Irrigators = 3200 in/40 in = 80 cfs * 1.983 af/d/cfs = 158.6 af/d * 62 days = **9836 acre-feet**

Residents = 2750 gpm/448.8 gpm/cfs = 6.13 cfs*1.983 af/d/cfs = 12.2 af/d * 62 days = **753 acre-feet**

West-Slope Cutthroat = 7.5 cfs * 1.983 af/d/cfs * 62 d = 922 acre-feet

September 1 storage = (July storage + Inflows) - (Outflows)

(25,000 af + 1844 af) – (9836 af + 753 af + 922 af) = **15,333 acre-feet**

Water Measurement

- > headgates
- > flow measurement basics
- > rated devices
- > flumes and weirs
- > automated devices
- > manual measurements
- > sample problems

MCA 85-5-302



....All persons using water from any stream or ditch whereon a water commissioner is appointed are required to have <u>suitable</u> headgates at the point where the ditch taps a stream and shall also, at some <u>suitable</u> place on the ditch and as near the headgate as practicable, place and maintain a <u>proper</u> measuring box, weir, or other appliance for the measurement of the waters flowing in the ditch.

What is a suitable headgate?





"Suitable" Headgate

- > Can be shut-off
- > Services the range of flows necessary to meet water rights
- > Can be operated by one person



















Rock Headgate – not properly functioning





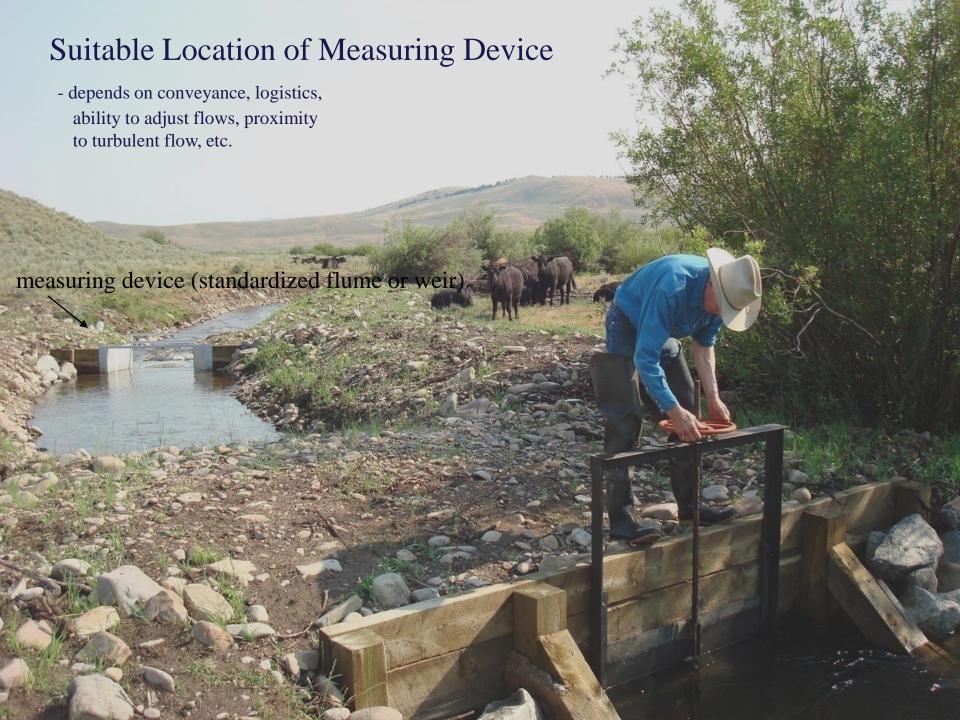


functional

not so functional











Flow Measurement Basics

Open Channel Flow

Closed Conduit Flow



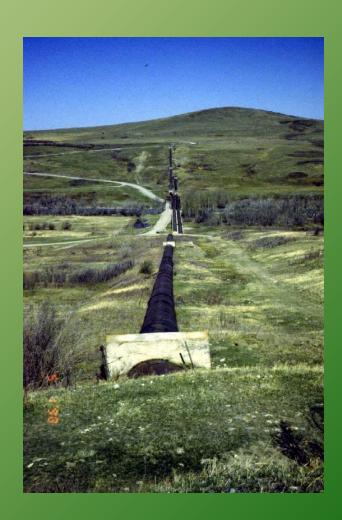


Open Channel Flow

- Occurs whenever the flowing stream has a free or unconstrained surface that is open to the atmosphere
- The force that causes flow is the force of gravity on the fluid
- Canals and streams
- Half-full pipe

Closed Conduit Flow

- Occurs when the conveyance conduit carries water under pressure
- No free surface open to the atmosphere
- Rate of discharge is a function of pressure or head difference between the inlet and the outlet
- Pipelines



Water Measurement Devices

> Rated and standard devices - staff gages, flumes, weirs, orifices, weir sticks

➤ Manual measurement - current meters, estimation techniques (float-area method)

Automated devices - gaging station, propeller meters, inline meters, ultra-sonic meters, totalizers

Rated Devices

Stage vs. Discharge Rating
Staff Gages
Flumes
Weirs
Weir Sticks

definitions

Stage - height of water surface above an established datum ex. staff gage reading

Discharge - volume of flow passing a point usually expressed in cubic feet per second (cfs) or inches.

Rating – relationship between the stage of the stream/canal and the discharge.





Staff Gages



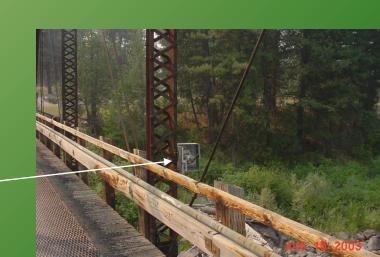


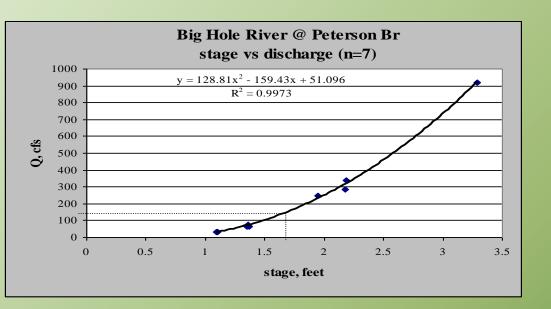


Stage = 1.16 feet

Stage = 0.67 feet

wire weight gage





<u>stage</u>	discharge	<u>stage</u>	discharge	<u>stage</u>	discharge
1.5	102	1.56	116	1.62	131
1.51	104	1.57	118	1.63	133
1.52	106	1.58	121	1.64	136
1.53	109	1.59	123	1.65	139
1.54	111	1.6	126	1.66	141
1.55	113	1.61	128	1.67	144

Table A8-12. Free-flow discharges in ${\rm ft}^3$ /sec through 1- to 8-foot Parshall flumes. Discharges for 2- to 8-ft flumes computed from the formula $Q=4.00Wh_a^{1.522(W^*0.026)}$. Discharges for 1-ft flume computed from the formula $Q=3.95h_a^{1.55}$.

Upper Head	4.0	D	ischarge fo	r flumes of					
ha, ft	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	
0.20	0.33	0.66	0.96	1.26					
.21	.35	.71	1.04	1.36	*****	*****			
.22	.38	.77	1.12	1.47					
.23	.40	.82	1.20	1.57		*****	*****	*****	
.24	.43	.88	1.28	1.68					
.25	.46	.93	1.37	1.80	2.22	2.63			
.26	.49	.99	1.46	1.91	2.36	2.80			
.27	.52	1.05	1.54	2.03	2.50	2.97		******	
.28	.55	1.11	1.63	2.15	2.65	3.15	*****	200000	
.29	.58	1.17	1.73	2.27	2.80	3.33	*****		
.30	.61	1.24	1.82	2.39	2.96	3.52	4.07	4.63	
.31	.64	1.30	1.92	2.52	3.12	3.71	4.29	4.88	
.32	.68	1.37	2.01	2.65	3.28	3.90	4.52	5.13	
.33	.71	1.44	2.11	2.78	3.44	4.10	4.75	5.39	
.34	.74	1.50	2.22	2.92	3.61	4.30	4.98	5.66	
.35	.78	1.57	2.32	3.05	3.78	4.50	5.21	5.92	
.36	.81	1.64	2.42	3.19	3.95	4.71	5.46	6.20	
.37	.85	1.71	2.53	3.33	4.13	4.92	5.70	6.48	
.38	.88	1.79	2.64	3.48	4.31	5.13	5.95	6.76	
.39	.92	1.86	2.75	3.62	4.49	5.35	6.20	7.05	
.40	.95	1.93	2.86	3.77	4.67	5.57	6.46	7.34	
.41	.99	2.01	2.97	3.92	4.86	5.79	6.72	7.64	
.42	1.03	2.09	3.08	4.07	5.05	6.02	6.98	7.94	
.43	1.07	2.16	3.20	4.22	5.24	6.25	7.25	8.25	
.44	1.11	2.24	3.32	4.38	5.43	6.48	7.52	8.56	
.45	1.15	2.32	3.44	4.54	5.63	6.72	7.80	8.87	
.46	1.19	2.40	3.56	4.70	5.83	6.96	8.08	9.19	
.47	1.23	2.48	3.68	4.86	6.03	7.20	8.36	9.51	
.48	1.27	2.57	3.80	5.03	6.24	7.45	8.65	9.84	
.49	1.31	2.65	3.93	5.19	6.45	7.69	8.94	10.2	
.50	1.35	2.73	4.05	5.36	6.66	7.95	9.23	10.5	
.51	1.39	2.82	4.18	5.53	6.87	8.20	9.53	10.5	
.52	1.43	2.90	4.31	5.70	7.08	8.46	9.83	11.2	
.53	1.48	2.99	4.44	5.88	7.30	8.72	10.1		
.54	1.52	3.08	4.57	6.05	7.52	8.98	10.1	11.5 11.9	
.55	1.56	3.17	4.71	6.23	7.74	9.25	10.8		
.56	1.61	3.26	4.84	6.41	7.97	9.52	11.1	12.2	
.57	1.65	3.35	4.98	6.59	8.20	9.52	11.1	12.6	
.58	1.70	3.44	5.11	6.77	8.43	10.1	11.4	13.0	
.59	1.74	3.53	5.25	6.96	8.66	10.1	12.0	13.3	
								13.7	

A-17













Accurate Water Measurement is dependent on:

- Measuring device selection
- Installation
- Correct use of measuring device
- Maintenance and quality control

Selection Criteria

- > Accuracy
- > Cost
- > Range of flows
- > Head loss
- > Adaptability to site conditions
- > Ability to pass sediment
- ➤ Maintenance requirements
- > Longevity of device for given environment
- User acceptance
- Vandalism potential
- > Impact on environment

Selecting a measuring device

- 1) Weir or Flume?
- 2) Which specific type of weir or flume?
- 3) What size?

Flumes and Weirs

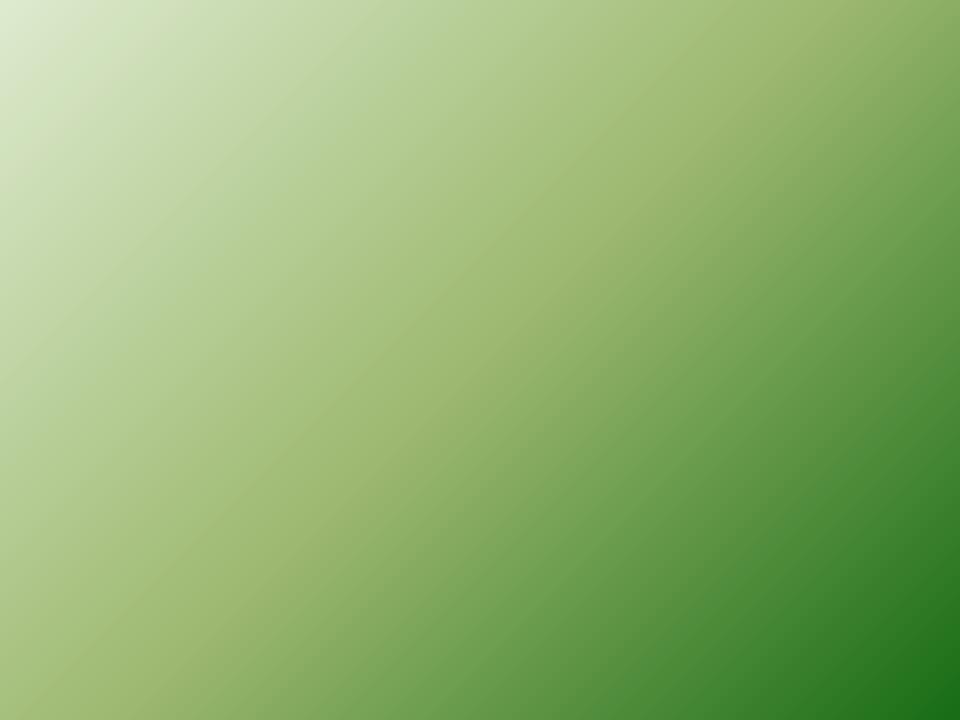
Flume – shaped, open-channel flow sections that force flow to accelerate.



Weir – an overflow structure built perpendicular to an open channel axis to measure the rate of flow.

Slope > 0.5%







Flumes

- Parshall
- Montana
- Cutthroat
- Ramp

Flume Classes

Long-Throated

Control discharge rate in a throat that is long enough to cause nearly parallel flow lines in the region of flow control. Ex. Ramp Flume



Control discharge in a region that produces curvilinear flow. Ex. Parshall Flume





Parshall Flume



- > low head loss requirement
- > facilitates sand and silt
- > tranquil flow (sub-critical) can be > 1 ft/s for approach
- > wide range of sizes and flows
- can be measured under some submerged conditions
- > difficult to build
- > installation accuracy critical
- > minimum head of 0.2 feet
- > expensive (2.5' throat = \$1500 to \$2500)

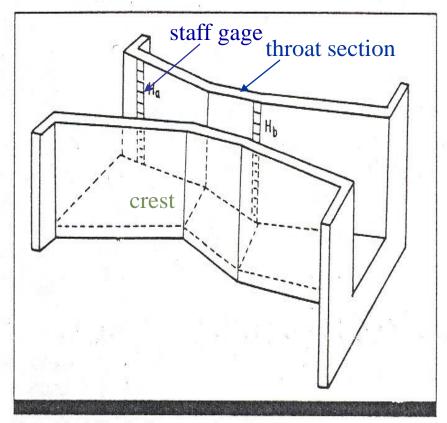


Figure 1. A Parshall measuring flume.

1 in. to 50 ft. (0.03 to > 3000 cfs)

2' Parshall 1 to 33 cfs

Parshall Flumes

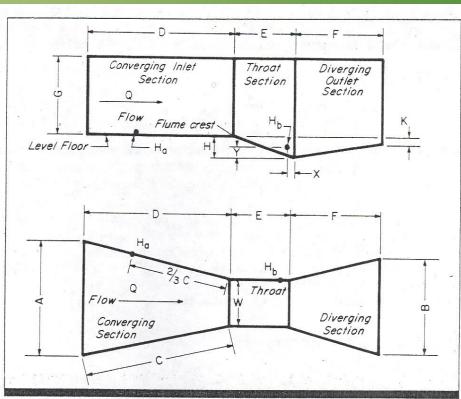


Figure 2. Section and plan views of a Parshall flume.



Specifications:

- > straight section of ditch
- > clear of obstructions that may disrupt even flow of approach
- > floor of converging section must be level lengthwise and cross wise
- > set flume floor above elevation of ditch to avoid submergence
- > staff gage set at floor of converging section (crest)
- > staff gage set 2/3 from crest









Throat width = 4 feet Q = ?Stage = 0.49 feet

Water Measurement Manual

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Technical Publication

U.S. Department of the Interior
Bureau of Reclamation
Third edition

Table A8-12. Free-flow discharges in ft³/sec through 1- to 8-foot Parshall flumes. Discharges for 2- to 8-ft flumes computed from the formula $Q = 4.00Wh_a^{1.522(W^{\circ}0.026)}$. Discharges for 1-ft flume computed from the formula $Q = 3.95h_a^{1.55}$.

Upper ha,		1.0	2.0	ischarge fo	or flumes of	various the	oat widths,	W 7.0	8.0
0.2	0	0.33	0.66					7.0	8.0
.2		.35	.71	0.96	1.26				*****
.2		.38	.77	1.04	1.36				*****
.2		.40		1.12	1.47	*****			*****
.2			.82	1.20	1.57			*****	*****
		.43	.88	1.28	1.68	*****	*****	*****	*****
.2		.46	.93	1.37	1.80	2.22	2.63		*****
.2		.49	.99	1.46	1.91	2.36	2.80	*****	*****
.2		.52	1.05	1.54	2.03	2.50	2.97		*****
.28		.55	1.11	1.63	2.15	2.65	3.15	****	*****
.29	9	.58	1.17	1.73	2.27	2.80	3.33		
.30		.61	1.24	1.82	2.39	2.96	3.52	4.07	4.63
.3		.64	1.30	1.92	2.52	3.12	3.71	4.29	4.88
.32	2	.68	1.37	2.01	2.65	3.28	3.90	4.52	5.13
.33	3	.71	1.44	2.11	2.78	3.44	4.10	4.75	5.39
.34	4	.74	1.50	2.22	2.92	3.61	4.30	4.98	5.66
.35	5	.78	1.57	2.32	3.05	3.78	4.50		
.36	3	.81	1.64	2.42	3.19	3.95	4.71	5.21	5.92
.37	7	.85	1.71	2.53	3.33	4.13	4.92	5.46 5.70	6.20
.38	3	.88	1.79	2.64	3.48	4.31	5.13		6.48
.39		.92	1.86	2.75	3.62	4.49	5.35	5.95 6.20	6.76 7.05
.40		.95	1.93	2.86	3.77				
.41		.99	2.01	2.97	3.92	4.67 4.86	5.57	6.46	7.34
.42		1.03	2.09	3.08	4.07		5.79	6.72	7.64
.43		1.07	2.16	3.20	4.07	5.05	6.02	6.98	7.94
.44		1.11	2.24	3.32	4.22	5.24	6.25 6.48	7.25	8.25
.45		1.15						7.52	8.56
.46		1.19	2.32	3.44	4.54	5.63	6.72	7.80	8.87
.40		1.19	2.40	3.56	4.70	5.83	6.96	8.08	9.19
.48		1.23	2.48	3.68	4.86	6.03	7.20	8.36	9.51
.40			2.57	3.80	5.03	6.24	7.45	8.65	9.84
	- 1	1.31	2.65	3.93	5.19	6.45	7.69	8.94	10.2
.50		1.35	2.73	4.05	5.36	6.66	7.95	9.23	10.5
.51		1.39	2.82	4.18	5.53	6.87	8.20	9.53	10.8
.52		1.43	2.90	4.31	5.70	7.08	8.46	9.83	11.2
.53		1.48	2.99	4.44	5.88	7.30	8.72	10.1	11.5
.54		1.52	3.08	4.57	6.05	7.52	8.98	10.4	11.9
.55		1.56	3.17	4.71	6.23	7.74	9.25	10.8	12.2
.56		1.61	3.26	4.84	6.41	7.97	9.52	11.1	12.6
.57		1.65	3.35	4.98	6.59	8.20	9.79	11.4	13.0
.58	- 1	1.70	3.44	5.11	6.77	8.43	10.1	11.7	13.3
.59		1.74	3.53	5.25	6.96	8.66	10.1	12.0	13.3

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0.20 .21 .22	0.33	and the second	3.0	4.0	5.0	6.0	7.0	8.0
.21		0.66	0.96	1.26				
	.35	.71	1.04	1.36				*****
	.38	.77	1.12	1.47			******	*****
.23	.40	.82	1.12					*****
.24	.43	.88		1.57	*****		*****	
			1.28	1.68	*****	*****		*****
.25	.46	.93	1.37	1.80	2.22	2.63		*****
.26	.49	.99	1.46	1.91	2.36	2.80		*****
.27	.52	1.05	1.54	2.03	2.50	2.97	*****	*****
.28	.55	1.11	1.63	2.15	2.65	3.15		*****
.29	.58	1.17	1.73	2.27	2.80	3.33	*****	
.30	.61	1.24	1.82	2.39	2.96	3.52	4.07	4.63
.31	.64	1.30	1.92	2.52	3.12	3.71	4.29	4.88
.32	.68	1.37	2.01	2.65	3.28	3.90	4.52	5.13
.33	.71	1.44	2.11	2.78	3.44	4.10	4.75	5.39
.34	.74	1.50	2.22	2.92	3.61	4.30	4.98	5.66
.35	.78	1.57	2.32	3.05	3.78	4.50		
.36	.81	1.64	2.42	3.19	3.95	4.71	5.21	5.92
.37	.85	1.71	2.53	3.33	4.13	4.92	5.46 5.70	6.20
.38	.88	1.79	2.64	3.48	4.31	5.13		6.48
.39	.92	1.86	2.75	3.62	4.49	5.35	5.95 6.20	6.76 7.05
.40	.95	1.93	2.86	3.77				
.41	.99	2.01	2.97	3.92	4.67	5.57	6.46	7.34
.42	1.03	2.09	3.08	4.07	4.86	5.79	6.72	7.64
.43	1.07	2.16	3.20	4.07	5.05	6.02	6.98	7.94
.44	1.11	2.24	3.32	4.22	5.24 5.43	6.25	7.25	8.25
.45						6.48	7.52	8.56
.45	1.15	2.32	3.44	4.54	5.63	6.72	7.80	8.87
.40	1.19 1.23	2.40	3.56	4.70	5.83	6.96	8.08	9.19
.48	1.23	2.48	3.68	4.86	6.03	7.20	8.36	9.51
.49		2.57	3.80	5.03	6.24	7.45	8.65	9.84
	1.31	2.65	3.93	5.19	6.45	7.69	8.94	10.2
.50	1.35	2.73	4.05	5.36	6.66	7.95	9.23	10.5
.51	1.39	2.82	4.18	5.53	6.87	8.20	9.53	10.8
.52	1.43	2.90	4.31	5.70	7.08	8.46	9.83	11.2
.53	1.48	2.99	4.44	5.88	7.30	8.72	10.1	11.5
.54	1.52	3.08	4.57	6.05	7.52	8.98	10.4	11.9
.55	1.56	3.17	4.71	6.23	7.74	9.25	10.8	12.2
.56	1.61	3.26	4.84	6.41	7.97	9.52	11.1	12.2
.57	1.65	3.35	4.98	6.59	8.20	9.79	11.4	13.0
.58	1.70	3.44	5.11	6.77	8.43	10.1	11.7	
.59	1.74	3.53	5.25	6.96	8.66	10.1	12.0	13.3 13.7



Q = 5.19 cfs



Rating Table = 5.19 cfs Measured flow (below) = 6.4 cfs



- > out of level
- > water flowing around or underneath
- > staff gage improperly set
- submerged condition





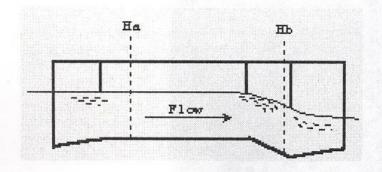


Figure 1

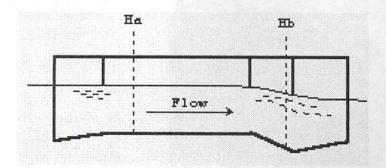
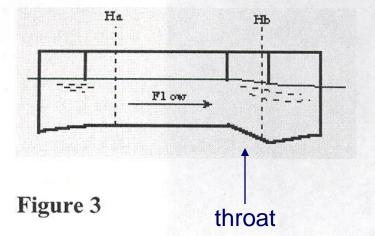


Figure 2



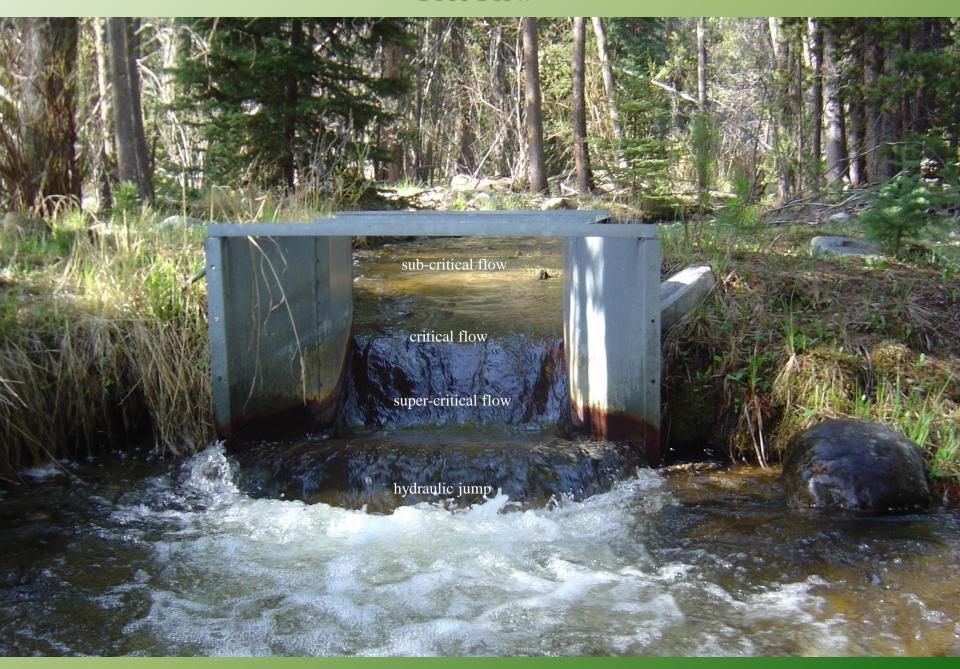
Free Flow

Defn. When the downstream water elevation does not influence flow through the measuring device.

Submerged Flow Determined by Ratio: H_b/H_a

Defn. Occurs when the downstream elevation of the water surface of the flume or weir is high enough to retard flow.

Free Flow





Submerged Parshall Flume Flow Calculation

University of Wyoming and US Bureau of Reclamation Methods

GIVEN

- · 4-foot throat width Parshall Flume
- Flume is level in both directions
- The upstream gage (Ha) reads 0.70 feet
- The flow through the flume appears to be submerged so a downstream gage (Hb) is installed
- The downstream gage (Hb) reads 0.59 feet

SUBMERGENCE DETERMINATION

Submergence is checked by finding the ratio of the downstream head to the upstream head, as shown below.

% submergence = (Hb / Ha) X 100

For our flume, the submergence is:

% submergence = (0.59 / 0.70) X 100

% submergence = 84%

Since the submergence is greater than 70%, this flow through this flume will have to be calculated as submerged flow. Please note that the % submergence requiring submergence calculations varies with the throat width of the flume. Check Page 13 in the Wyoming manual or Page 8-46 in the Water Measurement Manual for the maximum submergence allowed for free flow measurements.

WYOMING METHOD

Go to Figure 23 on Page 65. To use the figure it will be necessary to calculate the difference in the upstream (Ha) and downstream (Hb) heads.

Ha - Hb = 0.70 - 0.59 = 0.11 feet

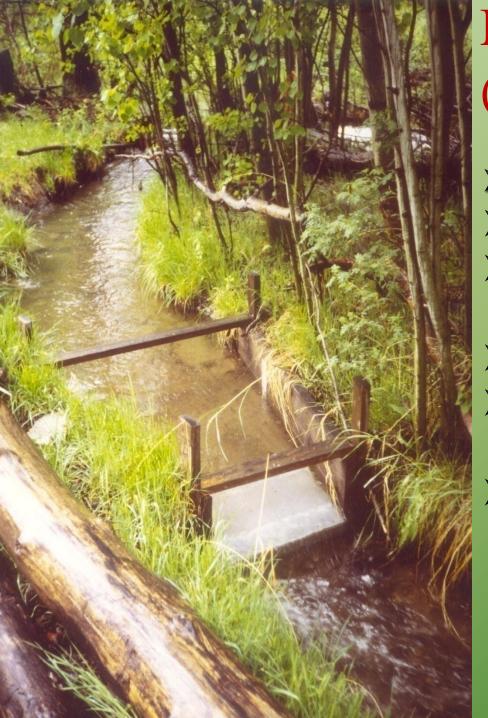
As shown in Illustration 1, start at 0.11 feet on the Ha — Hb axis (bottom). Move straight up until the 84% submergence line is met. From the intersection point on the 84% submergence line, move horizontally to the left until the discharge axis (left side) is crossed. Read the flume discharge of 7.95 cfs from the axis. Please note that this chart is valid for only a 4 foot throat Parshall Flume. Other charts, found in the manual, are required for other flume sizes.

Short-Throated Flumes

Parshall-Submergence

If Hb/Ha is less than percentage in table, then free flow exist and no submergence exists.

- 50% for 1,2, & 3 inch throats
- 60% for 6 & 9 inch throats
- 70% for 1 to 8 foot throats
- 80 % for 8 to 50 foot throats



Montana Flume (short parshall)

- > low head loss requirement
- > facilitates sediment
- > no approach velocity requirement
- > wide range of flows
- > easy to build
- cannot measure submergence, must have free flow







Cutthroat Flume

- > flat bottom
- > easy installation
- > less expensive than parshalls
- > easy to construct
- > variable hydraulic conditions
- > difficult to tell submergence







Long Throated Flumes

Ramp Flume
Replogle Flume
Broad-Crested Weir
(very similar)

Long-Throated Flumes













- •The required head loss over the long-throated flume to obtain a unique relationship between the upstream sill-referenced head and discharge is small.
- •Long-throated flumes can be made into portable devices that fit conveniently into open channels with considerably less complicated construction forming.

Pre-Fabricated Flumes

> example: 4 cfs capacity = \$650

Flume Inspection

- > Correct flume size
- > Check for free flow (no submergence)
- ➤ Floor of converging section (crest) is level crosswise and lengthwise
- > Staff gage is placed properly
- Check for seepage
- Clear of debris



Flume Field Inspection (parshall, ramp, cutthroat, Montana)

- Check level lengthwise and cross-wise.
- Check for free flow (outflow not influencing the elevation of inflow), an obvious drop in water level should appear downstream of the crest and a standing wave may be present.
- Make sure approach flow straight and relatively tranquil.
- Clean out sediment or debris that may be causing turbulence through inlet, throat, or outlet.



- > Make sure water does not flow around flume.
- Staff gage must be set on floor of converging section and 2/3 upstream of throat.
- Stage must be greater than 0.2 feet to function properly.

Contracted Weir Field Inspection (rectangular, cipoletti, V-notch)

- Check level on bulkhead and crest.
- Must have ventilated nappe for free flow conditions.
- Check for flow obstructions such as debris and sediment build-up and remove if necessary.
- Check for seepage around weir
- Approach velocity should appear relatively still (<0.5 feet per second).



- Notch plate should be plumb, smooth, and perpendicular to flow.
- Measuring point (bottom of staff gage) should be level with crest.
- H = maximum head expected. Crest must be 2H from sides, 3H from bottom, and 4H from measuring point (staff gage).
- Head measurement should be greater than 0.2 feet but less than 1/3 crest length. For example, if the maximum head expected is 0.5 feet, then the crest length should be at least 1.5 feet.



























Overflow structure installed perpendicular to flow



Weir vs. Flume





- > head loss requirement (need more slope for weirs)
- > weirs have approach velocity requirement
- > weirs can be easier to build
- > weirs can collect sediment and debris

(require more maintenance)

Sharp-Crested Weir

3 Standard Types

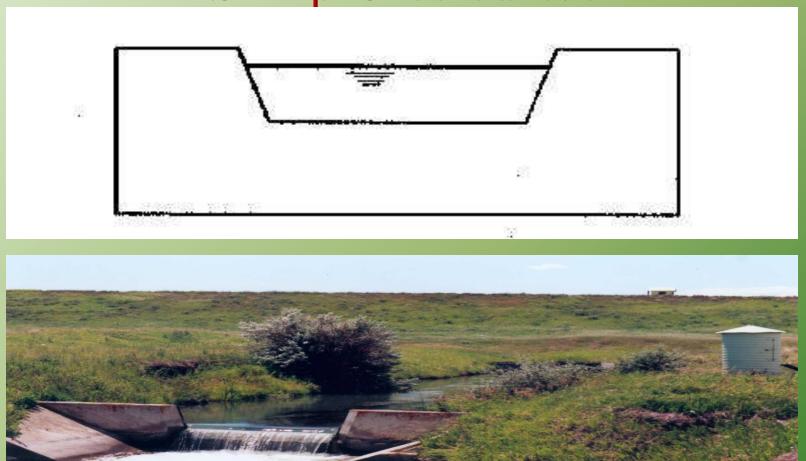
Contracted Rectangular
Cipolletti Contracted
Contracted Triangular or V-Notch

Sharp-Crested Weir



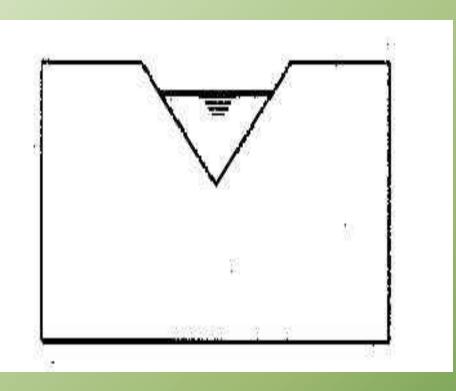
Contracted Rectangular

Sharp Crested Weir



Cipolletti Contracted - Trapezoidal in shape with sides that incline outwardly at a slope of 1 horizontal to 4 vertical. May be more accurate at lower stages than rectangular weir.

Sharp Crested Weir





Contracted Triangular or V-Notch

Measures flows up to 4.3 cfs or 1.25 feet of head

Conditions needed for all types of

Sharp-Crested Weirs

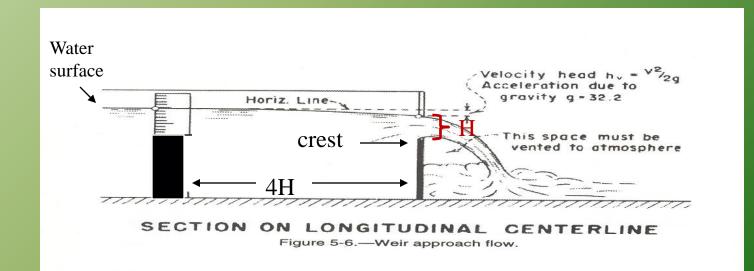
- Weir should be installed in straight section of ditch/canal.
- Upstream face of the weir plates and bulkhead should be plumb, smooth, and normal to the axis of the channel.
- Approach velocity <= 0.5 feet/second (appear still).





Weir Measurement

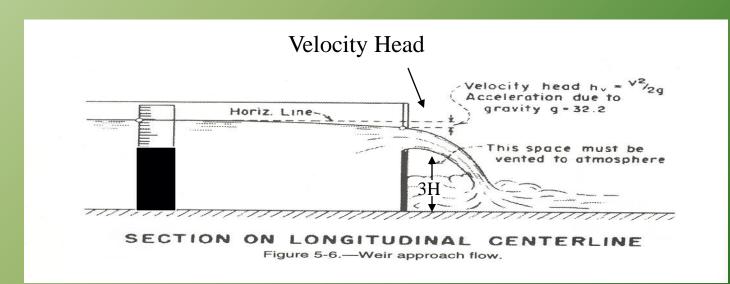
The measurement of head on the weir is the difference between the crest and the water surface at a point located upstream from the weir a distance of at least 4 times the maximum head on the crest (4H).





Free Flow Condition for Weirs

- The overflow sheet or nappe should touch only the upstream face of the crest and side plates (free flow).
- Maximum downstream water surface should be 3H from canal bottom.





Weir Installation Specifications



Weir Inspection and Maintenance

- > Approaching flow: low velocity (<0.5 f/s), even
- ➤ Check for flow obstructions, sediment, and/or debris build-up, remove if necessary
- > Check crest level
- > Check condition of crest
- ➤ Weir installation specifications (for contracted)
 - crest = 2H from sides
 - 3H from canal bottom
 - 4H from measuring point
- > Measuring point elevation = crest elevation
- \triangleright Adequate sizing (0.2 feet < H < 1/3 crest length)
- > Check for free flow conditions (nappe ventilation)
- > Check for seepage

Using your favorite Irrigation Water Measurement Guide, determine flow in cfs for the following measuring devices.

1) Parshall flume, throat width = 5 feet, gage reading

2) Montana Flume, throat width = 2.5 feet, gage reading

3) Cipoletti weir, crest length = 5 feet, gage reading

4) V-notch weir, gage reading

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet

Using your favorite Irrigation Water Measurement Guide, determine flow in cfs for the following measuring devices.

- 1) Parshall flume, throat width = 5 feet, gage reading gh = 1.16, Q = 25.3 cfs
- 2) Montana Flume, throat width = 2.5 feet, gage reading

3) Cipoletti weir, crest length = 5 feet, gage reading

4) V-notch weir, gage reading

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet

- 1) Parshall flume, throat width = 5 feet, gage reading gh = 1.16' Q = 25.3 cfs
- 2) Montana Flume, throat width = 2.5 feet, gage reading gh = 0.65' Q = 5.11 cfs
- 3) Cipoletti weir, crest length = 5 feet, gage reading

4) V-notch weir, gage reading

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet

- 1) Parshall flume, throat width = 5 feet, gage reading gh = 1.16' Q = 25.3 cfs
- 2) Montana Flume, throat width = 2.5 feet, gage reading gh = 0.65° Q = 5.11 cfs
- 3) Cipoletti weir, crest length = 5 feet, gage reading

$$gh = 0.51$$
' $Q = 6.13 cfs$

4) V-notch weir, gage reading

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet

- 1) Parshall flume, throat width = 5 feet, gage reading gh = 1.16' Q = 25.3 cfs
- 2) Montana Flume, throat width = 2.5 feet, gage reading gh = 0.65' Q = 5.11 cfs
- 3) Cipoletti weir, crest length = 5 feet, gage reading

$$gh = 0.51$$
' $Q = 6.13 cfs$

4) V-notch weir, gage reading

$$gh = 0.43$$
' $Q = 0.31 cfs$

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet

- 1) Parshall flume, throat width = 5 feet, gage reading gh = 1.16' Q = 25.3 cfs
- 2) Montana Flume, throat width = 2.5 feet, gage reading gh = 0.65° Q = 5.11 cfs
- 3) Cipoletti weir, crest length = 5 feet, gage reading

$$gh = 0.51$$
' $Q = 6.13 cfs$

4) V-notch weir, gage reading

$$gh = 0.43$$
' $Q = 0.31 cfs$

5) Parshall flume, throat size = 12 feet, gage reading = 0.05 feet flow too low to accurately measure

Most Common Ditch/Canal Measuring Devices in Montana

If properly installed, maintained and operated, the following are acceptable measuring devices for Water Commissioners:

Flumes

Weirs

Parshall

Contracted rectangular

Montana

Cipolleti

Ramp

V-Notch

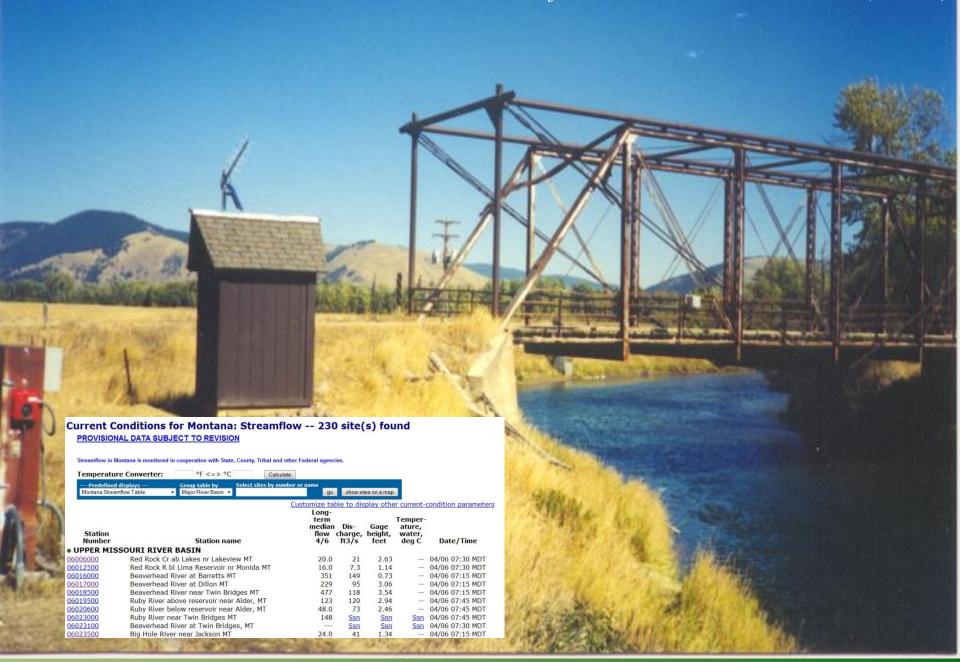
Cutthroat

Automated Devices

Streamflow Gaging Stations



Blackfoot River abv Nevada Creek (USGS)



Continuous Water Level Sensors

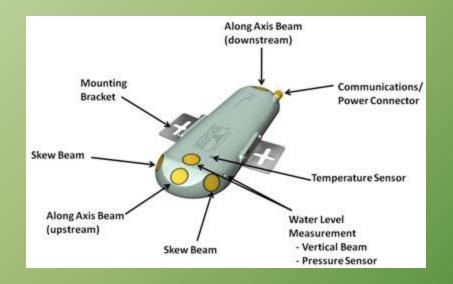
- TruTracks
- Pressure Transducers







Bottom Mounted Doppler Meters





In-Line Meters and Flow Totalizers







Ultra-Sonic Meters

Weir Sticks

• Commercially calibrated stick that shows depth of flow plus velocity head when placed on weir crest. In this case velocity head would be equal to the run up of water on the stick (Clausen Rule)

May be calibrated to be read at an angle.





- 1) Moderate to high head loss, low sediment load.
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions.
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs.
- 4) Low head loss, moderate to high sediment load, wide range of flows.
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible.
- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits.
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions.
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs.
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- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits.
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs.
- 4) Low head loss, moderate to high sediment load, wide range of flows.
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible.
- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits.
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows.
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible.
- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits.
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- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows. flume
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible.
- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows. flume
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible. bucket, stop watch
- 6) Water right is contract water that is administered based on volume.
- 7) A ditch has a number of standard outlets through pipe conduits.
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows. flume
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible. bucket, stop watch
- 6) Water right is contract water that is administered based on volume. flume with totalizer
- 7) A ditch has a number of standard outlets through pipe conduits.
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows. flume
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible. bucket, stop watch
- 6) Water right is contract water that is administered based on volume. flume, totalizer
- 7) A ditch has a number of standard outlets through pipe conduits. portable propeller, ultra sonic meter
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir.

- 1) Moderate to high head loss, low sediment load. weir
- 2) High sediment load, trapezoidal ditch, potential for submerged conditions. broad-crested weir or ramp flume
- 3) Need to measure seepage below a dam. Expected flows 0.5-2 cfs. V-notch or small rectangular weir
- 4) Low head loss, moderate to high sediment load, wide range of flows. flume
- 5) Point of diversion is small discharge (<25 gpm) from a pipe that is easily accessible. bucket, stop watch
- 6) Water right is contract water that is administered based on volume. flume, rated section, totalizer
- 7) A ditch has a number of standard outlets through pipe conduits. portable propeller, ultra sonic meter
- 8) DNRC hydrologist is on-site and you want to check accuracy of your weir. current meter

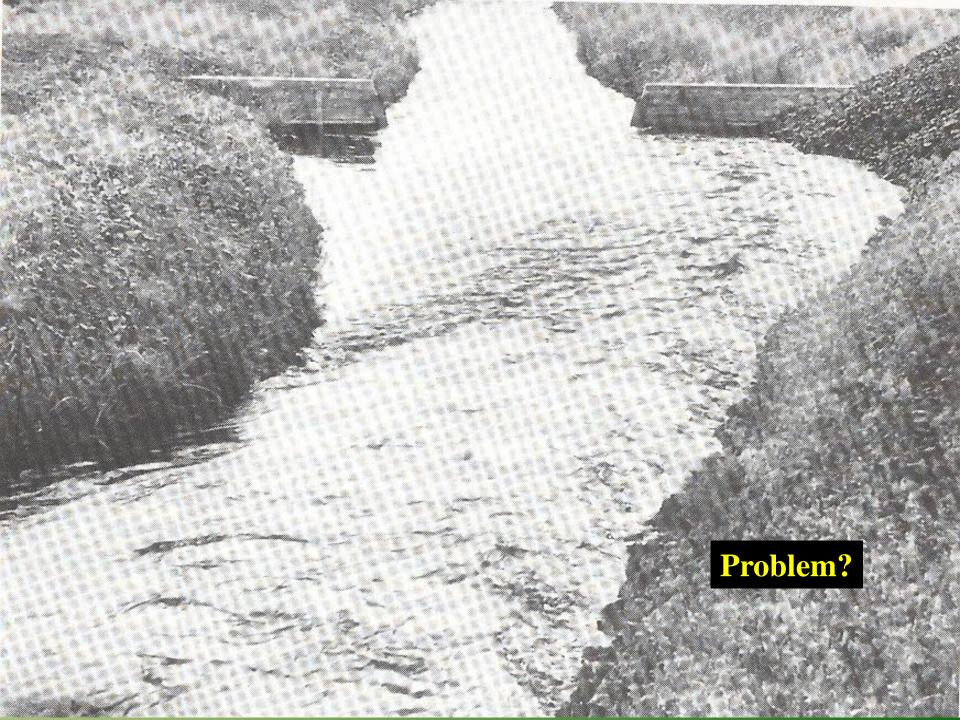
More Visual Examples

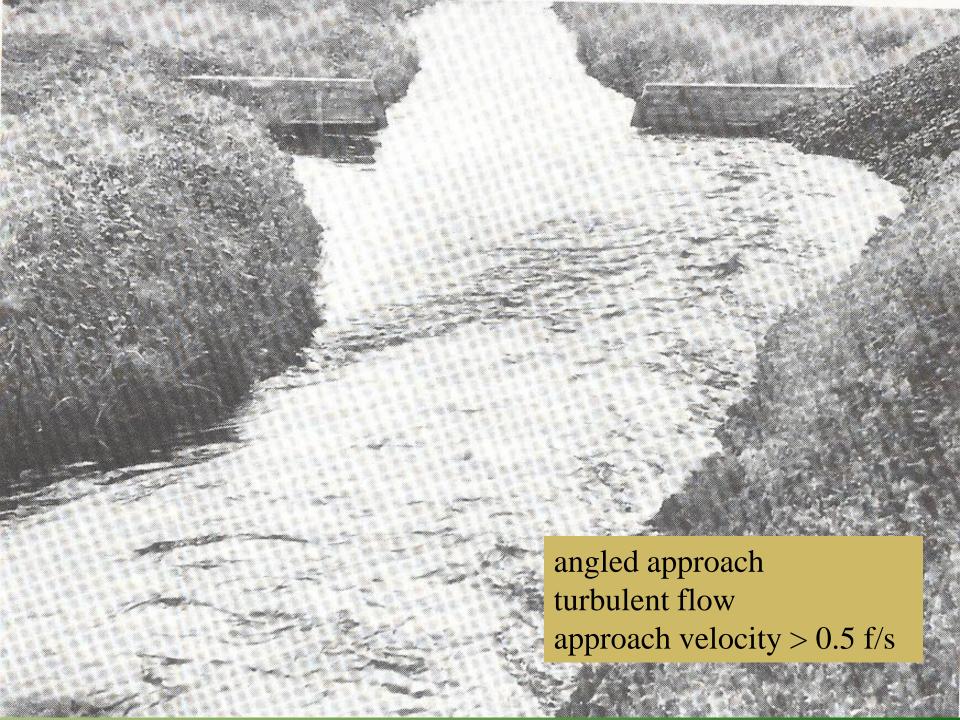
























































































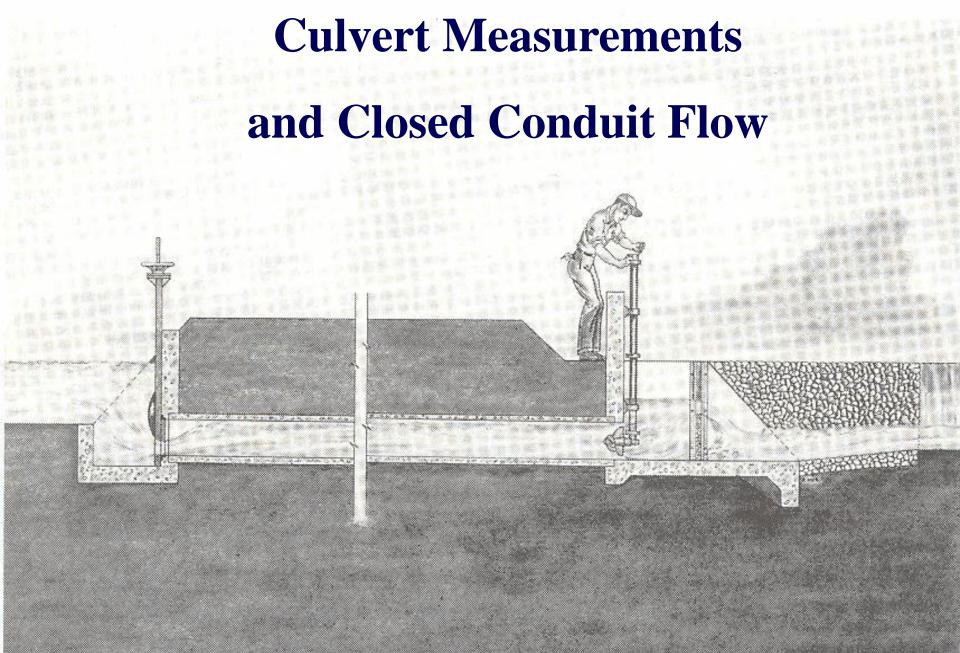


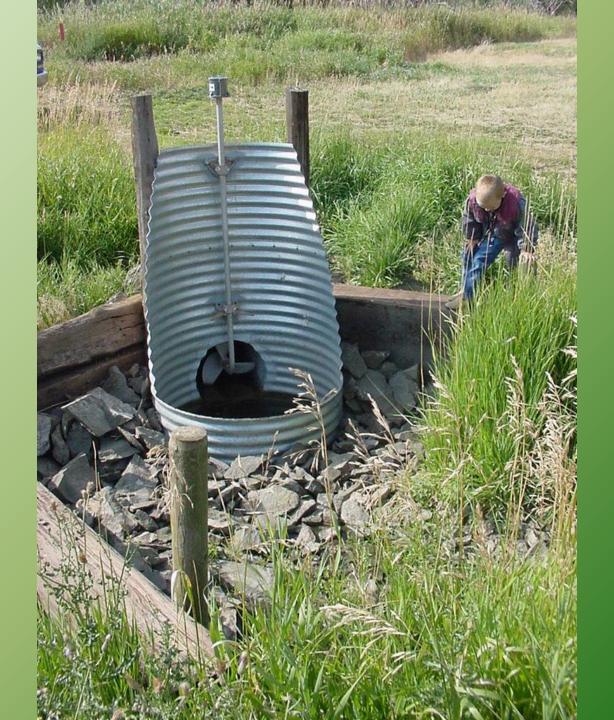






Closed Conduit Flow









Estimating Water Flow Rates

W.L. Trimmer



Increasing competition for water resources has made water conservation a high priority. Measuring the flow rate of water is the first step to good water management. All water right holders in the State of Oregon must be able to measure the flow rate of the water being diverted.

If a flow meter, flume, or weir isn't available, there are several methods available to estimate flow rate that you can do with available tools like stopwatches, rulers, and buckets.

The usual unit measuring flow rate for irrigation water rights is a cubic foot per second (cfs). This is water flowing through a cross-sectional area of 1 ft² at a velocity of 1 foot per second, and it's sometimes called a second-foot.

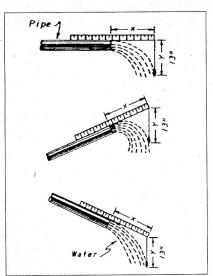


Figure 1.—Measuring horizontal distance (X) of a pipe flowing full with vertical drop Y=13".

A common diversion rate in eastern Oregon might be 1 cfs/40 acres. Here are some handy conversions (see page 4 for others): 1 cfs is about 450 gallons per minute; 1 cfs is about 1 acre-inch per hour; 1 cfs is about 2 acre-feet per day.

Propeller flow meters, weirs, and flumes provide the most accurate measures of flow rate, but in many instances you must make an estimate without them. Here are four methods to estimate irrigation diversions.

Method 1 Discharge from a pipe

If water can freely drop from a pipe, you can estimate the flow rate by measuring length with nothing more than a carpenter's rule. When the pipe is flowing full, place the rule as shown in Figure 1 and measure a horizontal distance when the vertical drop Y = 13 inches.

Find the proper pipe size in Table 1, and the discharge is in gallons per minute (gpm). If the pipe isn't level, use a plumb bob to measure the vertical drop Y.

Example 1. An 8-inch-diameter pipe is flowing full, and the horizontal distance X is measured to be 20 inches. From Table 1, the flow rate is 1.005 gpm.

If the pipe is flowing only partially full, find the ratio of the unfilled portion of pipe to the diameter of the pipe to estimate flow rate in gallons per minute, as shown in Table 2.

Example 2. A 10-inch-diameter pipe is flowing only partially full. The measured distance U is 2 inches. The ratio $U \div D$ in Table 2 is $2 \div 10 = 0.2$. The flow rate is 825 gpm.

Walter L. Trimmer, former Extension irrigation specialist, Oregon State University.

Table 1.—Discharge (gallons per minute) from pipes flowing full, with vertical drop Y = 13 $^{\prime\prime}$ and variable horizontal distances X.

Pipe size		Horizontal distance X (in inches)												
Inside diam.	Area (sq in)	12	14	16	18	20	22	24	26	28	30	32	34	36
2.0	3.14	38	44	50	57	63	69	75	82	88	94	100	107	113
2.5	4.91	59	69	79	88	98	108	118	128	137	147	157	167	177
3.0	7.07	85	99	113	127	-141	156	170	184	198	212	226	240	255
4.0	12.57	151	176	201	226	251	277	302	327	352	377	402	427	453
5.0	19.64	236	275	. 314	354	393	432	471	511	550	589	628	668	-707
6.0	28.27	339	396	452	509	565	622	678	735	792	848	905	961	1013
7.0	38.48	462	539	616	693	770	847	924	1000	1077	1154	1231	1308	1385
8.0	50.27	603	704	804	905	1005	1106	1206	1307	1408	1508	1609	1709	1810
9.0	63.62	763	891	1018	1145	1272	1400	1527	1654	1781	1909	2036	2163	2290
10.0	78.54	942	1100	1257	1414	1471	1728	1885	2042	2199	2356	2513	2670	2827
11.0	95.03	1140	1330	1520	1711	1901	2091	2281	2471	2661	2851	3041	3231	3421
12.0	113.10	1357	1583	1809	2036	2262	2488	2714	2941	3167	3393	3619	3845	4072

 $Q = \underbrace{3.61 \, AX}_{\mbox{\sqrt{Y}}} \mbox{ where:} \qquad A = \mbox{C ross-sectional area of discharge pipe in square inches} \\ \qquad X = \mbox{H orizontal distance in inches} \\ \qquad Y = \mbox{V rotical distance in inches}$

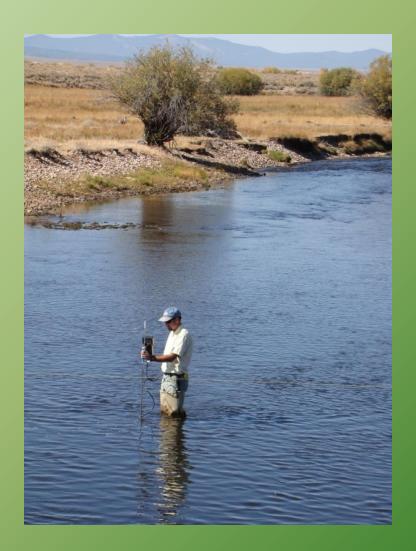
Table 2.—An approximate method of estimating discharge from pipes flowing partially full.

	Inside diameter of pipe = D in inches							
UD	4	6	8	10	12			
0.1	142	334	379	912	1310			
0.2	128	302	524	825	1185			
0.3	112	264	457	720	1034			
0.4	94	222	384	605	868			
0.5	75	176	305	480	689			
0.6	55	130	226	355	510			
0.7	37	88	152	240	345			
0.8	21	49	85	134	194			
0.9	8	17	30	52	74			
1.0	0	0	0	0	0			



Manual Measurements

- -Current meters
- Float-area method



Classes of current meters

- Mechanical: Anemometer and propeller velocity meters (not discussed)
- Electromagnetic velocity meters
- Doppler velocity meters

Electromagnetic

Example: Marsh-McBirney Velocity

Meter with digital read-out



Current meter probe produces a magnetic field, water moving through that field generates a voltage which is proportional to the velocity of the water



Maintenance (Marsh-McBirney)

- zero test every two weeks (depending on usage) or prior to going to field
- clean probe when necessary (400-600 grit sandpaper)
- May need laboratory calibration

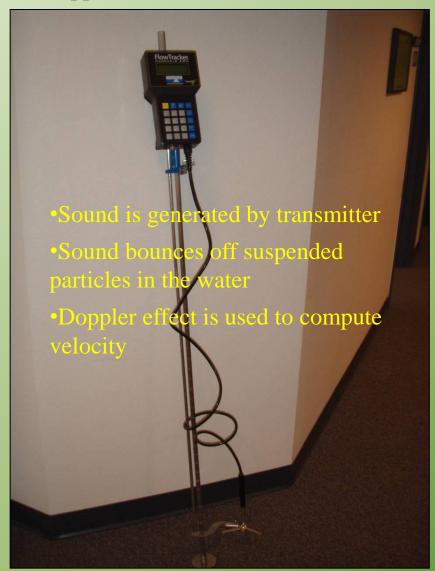


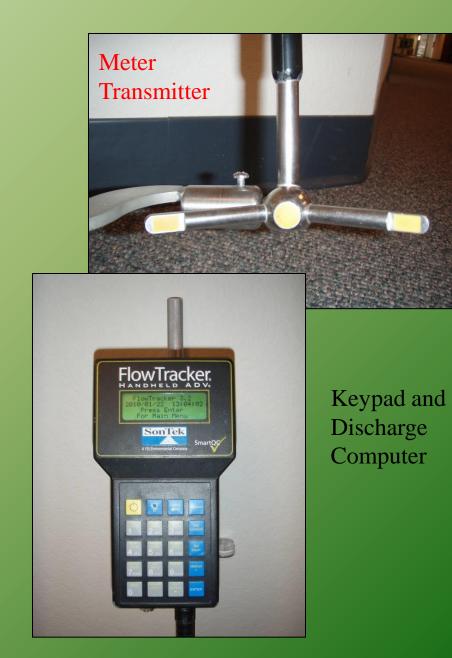


Doppler-Style Current Meters

Example: Flow-Tracker Acoustic

Doppler Meter





Doppler-Style Current Meters



Acoustic Doppler Current Profiler (ADCP)

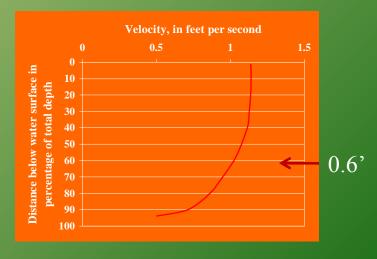
Measuring Flow with Current Meters

- Current meters measure velocity at a point.
- USGS Methodology

(Rantz, 1982 USGS WSP 2175 Nolan and Shields WRI 00-4036)

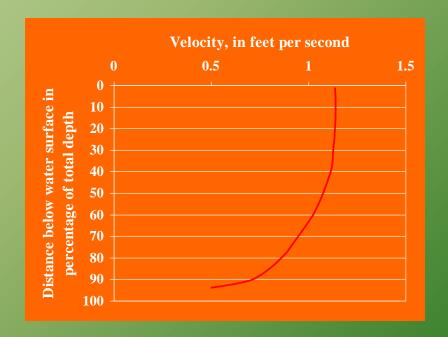
- Typically 20 points across section
 Accuracy Goal per section = 5%
 Re-measure if > 10%
- Meter is placed six-tenths depth from the surface (mean V)
- 40 second intervals





Measuring Flow with Current Meters

- If depth greater than 2.5 feet, 2-point measurement
 - average 0.2 and 0.8 depths
- If velocity profile is "abnormal", 3-point measurement
 - average 0.6 with average of 0.2 and 0.8

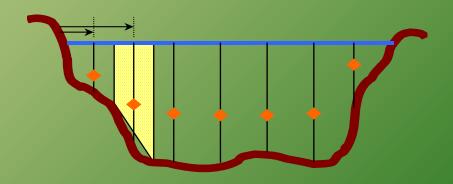


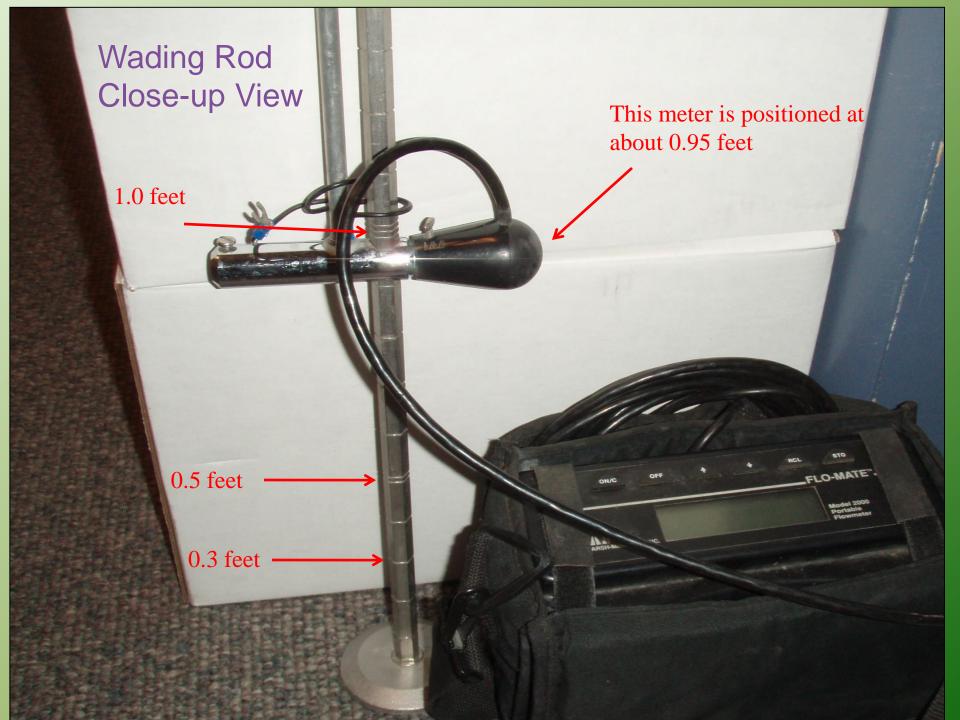
Velocity-Area principle used to compute discharge

 $Q = A \cdot V$

Total discharge is a summation of the partial discharges in measurement sections

$$Q_{Total} = A_1 \cdot V_1 + A_2 \cdot V_2 + \ldots + A_n \cdot V_n$$







Technique: Hold rod perpendicular to channel bottom
Hold instrument parallel to current
Stand behind and to the side of probe
Wear a cool hat

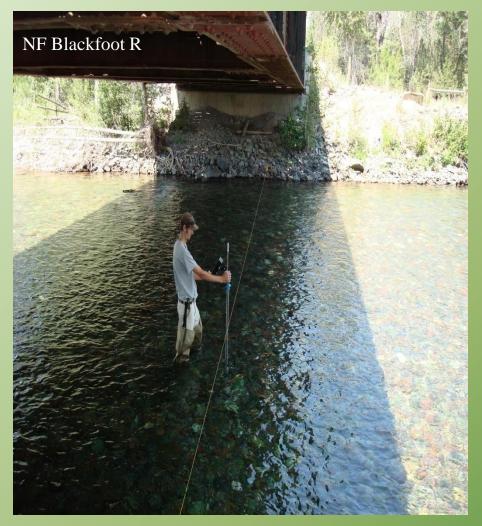
Selection of cross section for conventional current metering

- Cross section should lie within a straight reach, where stream flow lines are parallel to each other
- Velocities should be greater than 0.25 ft/s and depths greater than 0.25 ft
- Streambed should be relatively uniform and free of numerous boulders and heavy aquatic growth

Selection of cross section for conventional current metering (cont)

- Flow in cross section should be relatively uniform and free of eddies, slack water, and excessive turbulence
- Measurement section should be relatively close to the gaging station; there should be no tributary inflows or water diversions between the measurement section and the gage

Site Selection - Q





Good cross-section

Bad cross-section



Float-Area Method

Advantages

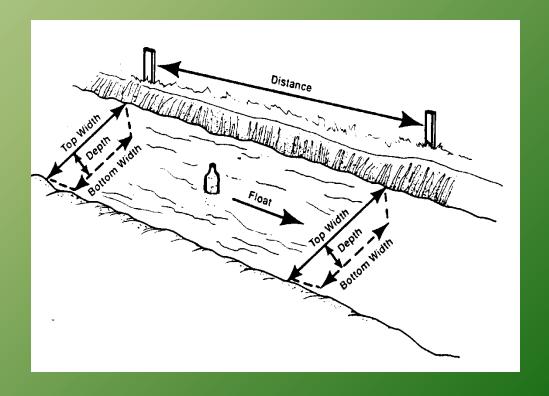
- Useful when elaborate methods not warranted (ballpark assessment)
- Useful for demonstrating flow-area concept
- Recognized by DNRC as estimation in water right physical availability analysis

Disadvantages

- difficulty in determining average cross section
- susceptible to wind currents, surface disturbances, and cross currents
- least accurate of all other methods, not applicable for enforcement
- Susceptible to criticism in a legal proceeding.

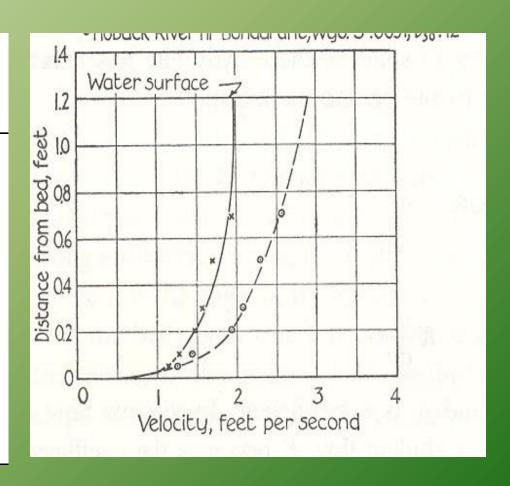
Float-Area Method

Utilizes Basic Flow Equation to determine discharge



Float-Area Method

Coefficients for C Float Velocity to Wa	•
Average Depth (ft)	_
1	0.66
2	0.68
3	0.70
4	0.72
5	0.74
6	0.76
9	0.77
12	0.78
15	0.79
20 and above	0.80



water commissioner (emperioneed)water commissioner (new time year) other	water commissioner	(experienced)	water commissioner	(new this year)	other
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1) Assuming the priority dates are the same, which purpose of use gets delivered first?

stock irrigation municipal instream flow for fisheries all at the same time

- 2) An irrigator is using a junior water right that is not in your District Court decree ahead of senior users that are in your decree. What course of action can you take to ensure water is properly diverted in priority?
- 3) An irrigator has a water right for 10 cfs out of Willow Creek. By the time water travels down a leaky ditch to their field, only 5 cfs remains. What is the maximum amount of water you, as water commissioner, can divert from Willow Creek?
- 4) A 2' parshall flume reads 1.64'. How much water is this equal to in cfs? In miner inches?
- 5) Name two things you would check when assessing the proper functioning of a flume or weir in the field?
- 6) What course of action would you take as a water commissioner if a water user's measuring device is not properly functioning?

